

APPENDIX D

SUPPLEMENTAL TRANSPORTATION ANALYSIS

Memorandum

To: Judy Malamut, LSA Associates, Inc.
From: Matt Weir, P.E., T.E., PTOE
Re: **Supplemental Traffic Analysis**
Dixon Ranch – El Dorado Hills, California
Date: May 15, 2015

I am writing to provide an overview of the supplemental analysis we recently completed for the above reference project. At the time of scoping for the project's traffic impact analysis in September 2011, El Dorado County's traffic analysis procedures dictated the consideration for Cumulative (2025) Conditions. At that time, the *General Plan* horizon year and Travel Demand Model (TDM) projections were both based on year the 2025. The project was subsequently revised and the County prepared a supplemental Scope of Work in November 2012 in which the requirement for consideration of the "same scenarios as the previous...study" was stated. Accordingly, the Draft EIR traffic study documents the traffic impact analysis results under year 2025 Cumulative Conditions.

This memorandum was prepared primarily in response to comment A5-1 received on the Draft EIR. In this comment, Caltrans expresses that the Cumulative (2025) analysis is "inadequate to accurately account for future cumulative impacts, especially given this project is not consistent with the adopted General Plan. Cumulative condition scenarios should be for year 2035 to match the latest future traffic model for El Dorado County." As a result, the following discussion presents the technical findings associated with our recent completion of a new Cumulative (2035) analysis for the proposed project.

Please note that this Cumulative (2035) analysis used the County's current TDM which is commonly referred to as the new "75-25" model in which 75 percent of the anticipated growth occurs within the Community Regions. Per the County's direction, Draft 2035 Land Use Forecast Data dated September 5, 2014, was incorporated. Also, in an effort to remain consistent with the original study³, the Cumulative (2035) roadway network assumptions inherent to these supplemental analyses do not reflect current SACOG MTP or El Dorado County CIP projects (i.e., US-50/Silva Valley Parkway Interchange Phase 2¹ and Green Valley Road widening between Francisco Drive and El Dorado Hills Boulevard/Salmon Falls Road²) that are understood to be reflected in recent studies (by others). While it is acknowledged that inclusion of these future projects would likely improve operations and even eliminate one or more significant impacts, we elected to maintain more restrictive and conservative geometric assumptions without these projects, thereby ensuring consistency between these analyses and the original study.

This supplemental analysis includes the addition of US-50 freeway facilities to the previously evaluated intersection facilities³, and documents Existing plus Approved Projects (2018 without and with the Proposed Project) and Cumulative (2035 without and with the Proposed Project) conditions. As documented in the original study³, because at least a portion of the Existing plus Approved Projects (2018) analysis scenarios relies on Cumulative forecasts for volume development, the 2018 analysis scenarios were updated with this supplemental analysis. In addition, because it was not previously

¹ *Adopted 2014 Capital Improvement Program*, Project No. 71345, El Dorado County Community Development Agency.

² *Adopted 2014 Capital Improvement Program*, Project No. GP178, El Dorado County Community Development Agency.

³ *Dixon Ranch Traffic Impact Analysis*, Kimley-Horn and Associates, Inc., June 18, 2013.

documented, this supplemental analysis also includes documentation of Existing (2014 without and with the Proposed Project) freeway facility conditions. The following is a brief discussion of the outcome of these supplemental technical analyses:

Intersections

- Existing plus Approved Projects (2018) Conditions
 - Results are consistent with the approved TIA³
 - Intersection #2 (Green Valley Road @ El Dorado Hills Boulevard)
 - Intersection #4 (Green Valley Road @ Loch Way)
- Cumulative (2035) Conditions
 - Results are consistent with the approved TIA³
 - Intersection #2 (Green Valley Road @ El Dorado Hills Boulevard)
 - Intersection #4 (Green Valley Road @ Loch Way)
 - Results eliminate impacts and mitigations at:
 - Intersection #7 (Green Valley Road @ Deer Valley Road)
 - Intersection #24 (Silva Valley Parkway @ Appian Way)

Freeway Facilities

- Existing (2014) Conditions
 - No impacts or mitigations required
- Existing plus Approved Projects (2018) Conditions
 - No impacts or mitigations required
- Cumulative (2035) Conditions
 - No impacts or mitigations required

We understand that Caltrans District 3 prefers weaving sections to be analyzed using the Leisch Method⁴ rather than Highway Capacity Manual (HCM) methodology. Rather than basing LOS on the density of the freeway mainline (HCM methodology), the Leisch Method focuses on the relationship between the length of the weaving section, the total weaving volume, and the speed of the vehicles in the weaving segment. Employing the Leisch Method (see **Table 5**) reveals that the critical weave section (westbound, west of El Dorado Hills Boulevard/Latrobe Road) would operate at LOS C during the AM peak-hour with and without the addition of the proposed project.

Exhibit 1 summarizes the results for these supplemental analyses. The attached tables and figures provide a detailed summary of the analysis results and the peak-hour traffic volumes.

Exhibit 1 – Impact Summary

Scenario	# Impacts	
	Intersections	Freeway ⁺
Existing (2014)	See Approved TIA ³	0
EPAP (2018, using 2035 Cumulative)	2, 0, 0	0
Cumulative (2035, 75-25 model)	2, -2, 0	0
Notes: Intersection impact results are presented in #, #, # format (# consistent, # eliminated, # new) ⁺ # new only because facilities were not studied in approved TIA		

⁴ Procedure for Analysis and Design of Weaving Sections, Federal Highway Administration, February 1984.

As indicated in **Exhibit 1**, the results of this new 2035 analysis determined the following:

- Three fewer intersection impacts when compared to the 2025 conditions documented in the Draft EIR.
- Although not originally studied in the Draft EIR traffic study, confirmation of no impacts to US-50 freeway facilities

Attachments to this memorandum include the worksheets for the technical analyses discussed herein.

Attachments:

Table 1 – Existing (2014) Freeway Levels of Service

Table 2 – Existing plus Approved Projects (2018) Intersection Levels of Services

Table 3 – Existing plus Approved Projects (2018) Freeway Levels of Services

Table 4 – Cumulative (2035) Intersection Levels of Service

Table 5 – Cumulative (2035) Freeway Levels of Service

Figure 1 – Existing plus Approved Projects (2018) Peak-Hour Volumes

Figure 2 – Existing plus Approved Projects (2018) plus Proposed Project Peak-Hour Volumes

Figure 3 – Cumulative (2035) Peak-Hour Volumes

Figure 4 – Cumulative (2035) plus Proposed Project Peak-Hour Volumes

Attachment A: Existing (2014) Freeway Analysis Worksheets

Attachment B: Existing plus Approved Projects (2018) Analysis Worksheets

Attachment C: Cumulative (2035) Analysis Worksheets

Table 1 – Existing (2014) Freeway Levels of Service

US-50				Existing (2014)		Existing (2014) Plus Proposed Project	
	Segment	Type	Peak Hour	Density ^a	LOS	Density ^a	LOS
Eastbound	West of Latrobe Rd SB Off Ramp	Basic	AM	12.3	B	12.3	B
			PM	21.4	C	21.4	C
	Latrobe SB Off Ramp	Diverge	AM	22.3	C	22.3	C
			PM	32.4	D	32.7	D
	Latrobe NB Off Ramp	Diverge	AM	12.0	B	12.0	B
			PM	26.6	C	26.8	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	Basic	AM	4.7	A	4.6	A
			PM	11.8	B	11.6	B
	Latrobe Rd On Ramp	Merge	AM	13.0	B	12.9	B
			PM	24.3	C	24.1	C
East of Latrobe Rd On Ramp	Basic	AM	6.9	A	6.8	A	
		PM	16.5	B	16.2	B	
Westbound	East of El Dorado Hills Blvd Off Ramp	Basic	AM	30.4	D	30.4	D
			PM	15.4	B	15.4	B
	El Dorado Hills Blvd Off Ramp	Diverge	AM	36.3	E	36.3	E
			PM	22.4	C	22.4	C
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	Basic	AM	20.2	C	20.2	C
			PM	11.0	A	11.0	A
	El Dorado Hills Blvd On Ramp	Merge	AM	36.7	E	37.0	E
			PM	27.8	C	28.0	D
West of El Dorado Hills Blvd On Ramp	Basic	AM	43.9	E	44.9	E	
		PM	26.7	D	27.0	D	

Notes:

- a- Density measured in passenger cars/lane/mile (pc/ln/mi)
- b- **Bold** represents unacceptable operations. Shaded represents significant impact.

Table 2 – Existing plus Approved Projects (2018) Intersection Levels of Service

INTERSECTION		PEAK HOUR	2025 MODEL				2035 (75-25) MODEL			
			EPAP BASELINE		EPAP PLUS PROJECT		EPAP BASELINE		EPAP PLUS PROJECT	
			DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)	
1	Green Valley Rd. & Francisco Rd. *	AM	33.0	C	34.6	C	33.0	C	34.6	C
		PM	32.8	C	34.6	C	32.8	C	34.6	C
2	El Dorado Hills Blvd. & Green Valley Rd. *	AM	83.7	F	108.0	F	83.7	F	108.0	F
		PM	78.7	E	108.1	F	78.7	E	108.1	F
3	Silva Valley Pkwy. & Green Valley Rd. *	AM	33.6	C	46.7	D	33.6	C	46.7	D
		PM	25.8	C	42.9	D	25.8	C	42.9	D
4	Loch Way & Green Valley Rd *	AM	24.0	C	36.8	E	24.0	C	36.8	E
		PM	32.3	D	60.6	F	32.3	D	60.6	F
5	Green Valley Rd & Wilson Estates *	AM	17.7	C	24.8	C	17.7	C	24.8	C
		PM	17.6	C	26.1	D	17.6	C	26.1	D
6	Green Valley Rd & Malcom Dixon Rd *	AM	16.5	C	22.4	C	16.5	C	22.4	C
		PM	19.8	C	30.8	D	19.8	C	30.8	D
7	Deer Valley Rd. & Green Valley Rd. *	AM	20.4	C	22.7	C	20.4	C	22.7	C
		PM	25.2	D	29.0	D	25.2	D	29.0	D
8	Silver Springs Pkwy & Green Valley Rd *	AM	8.5	A	8.9	A	8.4	A	9.0	A
		PM	7.8	A	8.1	A	7.8	A	8.4	A
9	Bass Lake Rd. & Green Valley Rd. ***	AM	22.1	C	23.3	C	20.4	C	21.4	C
		PM	22.3	C	23.0	C	20.9	C	21.3	C
10	Cambridge Rd. & Green Valley Rd. ***	AM	18.6	B	19.3	B	18.3	B	19.0	B
		PM	21.0	C	22.1	C	19.5	B	20.2	C
11	Cameron Park Dr. & Green Valley Rd. ***	AM	27.6	C	30.8	C	30.3	C	33.0	C
		PM	31.7	C	34.9	C	29.6	C	32.1	C
12	El Dorado Hills Blvd. & Francisco Dr. *	AM	16.8	C	17.8	C	16.8	C	17.8	C
		PM	22.2	C	23.0	C	22.2	C	23.0	C
13	El Dorado Hills Blvd. & Harvard Way ***	AM	13.2	B	13.3	B	13.2	B	13.3	B
		PM	10.6	B	10.7	B	10.6	B	10.7	B
14	El Dorado Hills Blvd. & Serrano Pkwy. ***	AM	39.9	D	40.9	D	39.0	D	39.7	D
		PM	16.9	B	17.1	B	16.9	B	17.1	B
15	El Dorado Hills Blvd. & Saratoga Wy. (North) *	AM	31.2	C	31.3	C	31.1	C	31.2	C
		PM	25.0	C	24.7	C	25.0	C	24.7	C
16	El Dorado Hills Blvd. & Saratoga Wy. (South) ***	AM	28.1	C	29.0	C	28.2	C	27.8	C
		PM	29.9	C	30.2	C	29.9	C	29.7	C
18	Latrobe Rd. & US-50 EB Ramp ***	AM	10.5	B	10.5	B	10.5	B	10.5	B
		PM	10.4	B	10.4	B	10.4	B	10.4	B

Table 2 – Existing plus Approved Projects (2018) Intersection Levels of Service (cont.)

INTERSECTION		PEAK HOUR	2025 MODEL				2035 (75-25) MODEL			
			EPAP BASELINE		EPAP PLUS PROJECT		EPAP BASELINE		EPAP PLUS PROJECT	
			DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)	
19	Silva Valley Pkwy & EB US-50 Ramps **	AM	18.1	C	18.5	B	18.1	B	18.5	B
		PM	34.7	C	37.7	D	34.7	C	37.7	D
20	Silva Valley Pkwy & WB US-50 Ramps **	AM	28.7	C	29.8	C	28.7	C	29.8	C
		PM	42.5	D	41.5	D	42.5	D	41.5	D
21	Silva Valley Pkwy & Country Club Dr. **	AM	9.7	A	9.8	A	9.7	A	9.8	A
		PM	7.7	A	8.2	A	7.7	A	8.2	A
22	Silva Valley Pkwy. & Serrano Pkwy. **	AM	46.5	D	49.0	D	46.5	D	49.0	D
		PM	42.6	D	44.0	D	42.6	D	44.0	D
23	Harvard Way & Silva Valley Pkwy. ***	AM	36.0	D	39.2	D	30.1	C	32.0	C
		PM	17.3	B	17.7	B	16.3	B	15.6	B
24	Silva Valley Pkwy. & Appian Way ***	AM	19.0	C	28.9	D	13.1	B	16.5	C
		PM	22.9	C	42.5	E	13.5	B	18.8	C
25	Site Dwy RIRO & Green Valley Rd. *	AM	-	-	10.8	B	-	-	10.8	B
		PM	-	-	18.5	C	-	-	18.5	C
26	Site Dwy. Full/Site Dwy. & Green Valley Rd. *	AM	-	-	9.2	A	-	-	9.2	A
		PM	-	-	9.7	A	-	-	9.7	A

Notes:

* Intersection volumes based on existing counts plus manual addition of approved projects in area.

** Intersection volumes based on Silva Valley Interchange Traffic Operations Study.

*** Intersection volumes based on model run outputs.

(a) Delay refers to the average control delay for the entire intersection, measured in seconds per vehicle. At a two-way stop-controlled intersection, delay refers to the worst movement.

(b) LOS calculations are based on the methodology outlined in the 2000 Highway Capacity Manual and performed using Synchro 8

Table 3 – Existing plus Approved Projects (2018) Freeway Levels of Service

US-50				Existing plus Approved Projects		Existing Plus Approved Projects plus Proposed Project	
	Segment	Type	Peak Hour	Density ^a	LOS	Density ^a	LOS
Eastbound	West of El Dorado Hills Blvd	Basic	AM	11.5	B	11.6	B
			PM	21.9	C	22.4	C
	Latrobe Rd SB Off Ramp	Diverge	AM	24.0	C	24.1	C
			PM	34.2	D	34.6	D
	Latrobe Rd NB Off Ramp	Diverge	AM	12.3	B	12.6	B
			PM	26.3	C	26.9	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	Basic	AM	4.6	A	4.6	A
			PM	11.7	B	11.9	B
	Latrobe Rd On Ramp to Silva Valley Pkwy Off Ramp	Weave	AM	7.0	A	7.1	A
			PM	15.7	B	15.9	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy SB On Ramp	Basic	AM	6.0	A	6.0	A
			PM	13.5	B	13.5	B
Silva Valley Pkwy SB On Ramp	Merge	AM	19.0	B	19.2	B	
		PM	29.9	D	30.1	D	
East of Silva Valley Pkwy	Basic	AM	10.1	A	10.2	A	
		PM	20.1	C	20.1	C	
Westbound	East of Silva Valley Pkwy	Basic	AM	29.1	D	29.2	D
			PM	14.7	B	14.8	B
	Silva Valley Off Ramp	Diverge	AM	24.8	C	24.8	C
			PM	15.2	B	15.4	B
	Silva Valley Off Ramp to Silva Valley NB On Ramp	Basic	AM	21.1	C	21.1	C
			PM	7.7	A	7.7	A
	Silva Valley NB On Ramp to El Dorado Hills Blvd Off Ramp	Weave	AM	26.0	C	26.3	C
			PM	10.1	B	10.2	B
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	Basic	AM	20.3	C	20.5	C
			PM	8.1	A	8.2	A
	El Dorado Hills Blvd On Ramp	Merge	AM	36.0	E	36.6	E
			PM	21.6	C	22.0	C
West of El Dorado Hills Blvd	Basic	AM	35.5	E	36.5	E	
		PM	15.8	B	16.0	B	

Notes:

a- Density measured in passenger cars/lane/mile (pc/l/mi)

Table 4 – Cumulative (2035) Intersection Levels of Service

INTERSECTION		PEAK HOUR	2025 MODEL				2035 (75-25) MODEL			
			CUMUL BASELINE		CUMUL PLUS PROJECT		CUMUL BASELINE		CUMUL PLUS PROJECT	
			DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)	
1	Green Valley Rd. & Francisco Rd. ***	AM	35.9	D	37.9	D	38.2	D	41.0	D
		PM	37.7	D	40.5	D	44.4	D	46.1	D
2	El Dorado Hills Blvd. & Green Valley Rd. ***	AM	120.5	F	145.4	F	50.4	D	62.0	E
		PM	90.6	F	120.9	F	77.9	E	101.2	F
3	Silva Valley Pkwy. & Green Valley Rd. ***	AM	45.9	D	65.8	E	29.5	C	37.1	D
		PM	35.6	D	53.4	D	21.5	C	32.0	C
4	Loch Way & Green Valley Rd ***	AM	26.5	D	42.3	E	23.8	C	51.1	F
		PM	35.4	E	70.3	F	62.1	F	275.0	F
5	Green Valley Rd & Wilson Estates ***	AM	19.2	C	27.4	D	14.6	B	19.6	C
		PM	19.0	C	28.8	D	14.2	B	19.6	C
6*	Green Valley Rd & Malcom Dixon Rd ***	AM	17.2	C	23.7	C	13.7	B	18.0	C
		PM	20.6	C	32.7	D	15.4	C	22.5	C
7*	Deer Valley Rd. & Green Valley Rd. ***	AM	21.1	C	23.6	C	19.3	C	21.9	C
		PM	37.2	E	46.1	E	24.4	C	29.1	D
8*	Silver Springs Pkwy & Green Valley Rd ***	AM	10.3	B	11.0	B	8.0	A	8.4	A
		PM	9.3	A	10.4	B	7.5	A	7.9	A
9	Bass Lake Rd. & Green Valley Rd. ***	AM	26.8	C	28.7	C	37.8	D	42.2	D
		PM	25.8	C	26.5	C	30.6	C	32.0	C
10	Cambridge Rd. & Green Valley Rd. ***	AM	21.7	C	22.2	C	37.5	D	41.9	D
		PM	25.3	C	27.9	C	58.0	E	63.6	E
11	Cameron Park Dr. & Green Valley Rd. ***	AM	32.1	C	35.6	D	38.2	D	42.6	D
		PM	38.4	D	43.0	D	43.2	D	47.2	D
12	El Dorado Hills Blvd. & Francisco Dr. ***	AM	17.3	C	18.5	C	39.3	E	45.5	E
		PM	22.5	C	23.3	C	34.8	D	39.4	E
13	El Dorado Hills Blvd. & Harvard Way ***	AM	16.1	B	16.2	B	31.2	C	31.7	C
		PM	10.4	B	10.5	B	14.3	B	14.5	B
14	El Dorado Hills Blvd. & Serrano Pkwy. ***	AM	51.3	D	54.1	D	77.3	E	79.0	E
		PM	19.9	B	20.3	C	52.3	D	51.7	D
15	El Dorado Hills Blvd. & Saratoga Wy. (North) ***	AM	32.4	C	34.2	C	27.7	C	28.7	C
		PM	51.8	D	51.5	D	52.6	D	51.7	D
16	El Dorado Hills Blvd. & Saratoga Wy. (South) ***	AM	33.9	C	33.5	C	33.9	C	33.7	C
		PM	43.9	D	43.9	D	25.6	C	25.7	C
18	Latrobe Rd. & US-50 EB Ramp **	AM	17.0	B	17.0	B	17.0	B	17.0	B
		PM	24.4	C	24.3	C	17.9	B	17.9	B

Table 4 – Cumulative (2035) Intersection Levels of Service (cont.)

INTERSECTION		PEAK HOUR	2025 MODEL				2035 (75-25) MODEL			
			CUMUL BASELINE		CUMUL PLUS PROJECT		CUMUL BASELINE		CUMUL PLUS PROJECT	
			DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)		DELAY (a) / LOS (b)	
19	Silva Valley Pkwy & EB US-50 Ramps **	AM	28.4	C	28.9	C	28.4	C	28.9	C
		PM	53.2	D	70.5	E	30.9	C	47.4	D
20	Silva Valley Pkwy & WB US-50 Ramps **	AM	56.4	E	59.8	E	56.4	E	59.8	E
		PM	70.6	E	74.6	E	42.2	D	44.9	D
21	Silva Valley Pkwy & Country Club Dr. **	AM	16.1	B	15.4	B	16.1	B	15.4	B
		PM	16.2	B	17.5	B	8.5	A	9.4	A
22	Silva Valley Pkwy. & Serrano Pkwy. **	AM	53.8	D	57.4	E	53.8	D	56.4	E
		PM	60.1	E	63.0	E	42.6	D	44.2	D
23	Harvard Way & Silva Valley Pkwy. ***	AM	69.9	E	77.1	E	36.4	D	39.8	D
		PM	24.1	C	25.5	C	17.8	B	18.1	B
24	Silva Valley Pkwy. & Appian Way ***	AM	35.6	E	62.4	F	15.6	C	21.3	C
		PM	54.3	F	95.1	F	17.7	C	29.7	D
25	Site Dwy RIRO & Green Valley Rd. ***	AM	-	-	11.0	B	-	-	10.3	B
		PM	-	-	19.2	C	-	-	15.5	C
26	Site Dwy. Full/Site Dwy. & Green Valley Rd. ***	AM	-	-	10.9	B	-	-	9.2	A
		PM	-	-	12.6	B	-	-	9.6	A

Notes:

* Intersections 6, 7, and 8 are located outside Community Regions. LOS D is the minimum acceptable LOS for these intersections.

** Intersection volumes based on Silva Valley Interchange Traffic Operations Study.

*** Intersection volumes based on model run outputs.

(a) Delay refers to the average control delay for the entire intersection, measured in seconds per vehicle. At a two-way stop-controlled intersection, delay refers to the worst movement.

(b) LOS calculations are based on the methodology outlined in the 2000 Highway Capacity Manual and performed using Synchro 8

Table 5 – Cumulative (2035) Freeway Levels of Service

US- 50				Cumulative		Cumulative plus Proposed Project	
	Segment	Type	Peak Hour	Density ^a	LOS	Density ^a	LOS
Eastbound	West of El Dorado Hills Blvd	Basic	AM	13.7	B	13.9	B
			PM	25.6	C	26.1	D
	Latrobe Rd SB Off Ramp	Diverge	AM	27.2	C	27.3	C
			PM	37.1	E	37.4	E
	Latrobe Rd NB Off Ramp	Diverge	AM	14.4	B	14.6	B
			PM	29.1	D	29.8	D
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	Basic	AM	6.3	A	6.4	A
			PM	14.3	B	14.4	B
	Latrobe Rd On Ramp to Silva Valley Pkwy Off Ramp	Weave	AM	9.2	A	9.2	A
			PM	18.5	B	18.7	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy SB On Ramp	Basic	AM	7.6	A	7.6	A
			PM	16.1	B	16.1	B
	Silva Valley Pkwy SB On Ramp	Merge	AM	15.9	B	16.2	B
			PM	26.2	C	26.4	C
Silva Valley Pkwy NB On Ramp	Merge	AM	14.6	B	14.7	B	
		PM	23.3	C	23.4	C	
East of Silva Valley Pkwy	Basic	AM	12.5	B	12.6	B	
		PM	23.4	C	23.5	C	
Westbound	East of Silva Valley Pkwy	Basic	AM	27.4	D	27.5	D
			PM	15.9	B	16.0	B
	Silva Valley Off Ramp to Silva Valley NB On Ramp	Basic	AM	30.9	D	30.9	D
			PM	14.0	B	14.0	B
	Silva Valley NB On Ramp to El Dorado Hills Blvd Off Ramp	Weave	AM	28.4	D	28.7	D
			PM	13.5	B	13.6	B
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	Basic	AM	31.5	D	31.9	D
			PM	14.4	B	14.5	B
	West of El Dorado Hills Blvd	Weave	AM (HCM)	-	F	-	F
			AM (Leisch)	-	C	-	C
PM			18.9	B	18.9	B	

Notes:

a- Density measured in passenger cars/lane/mile (pc/ln/mi)

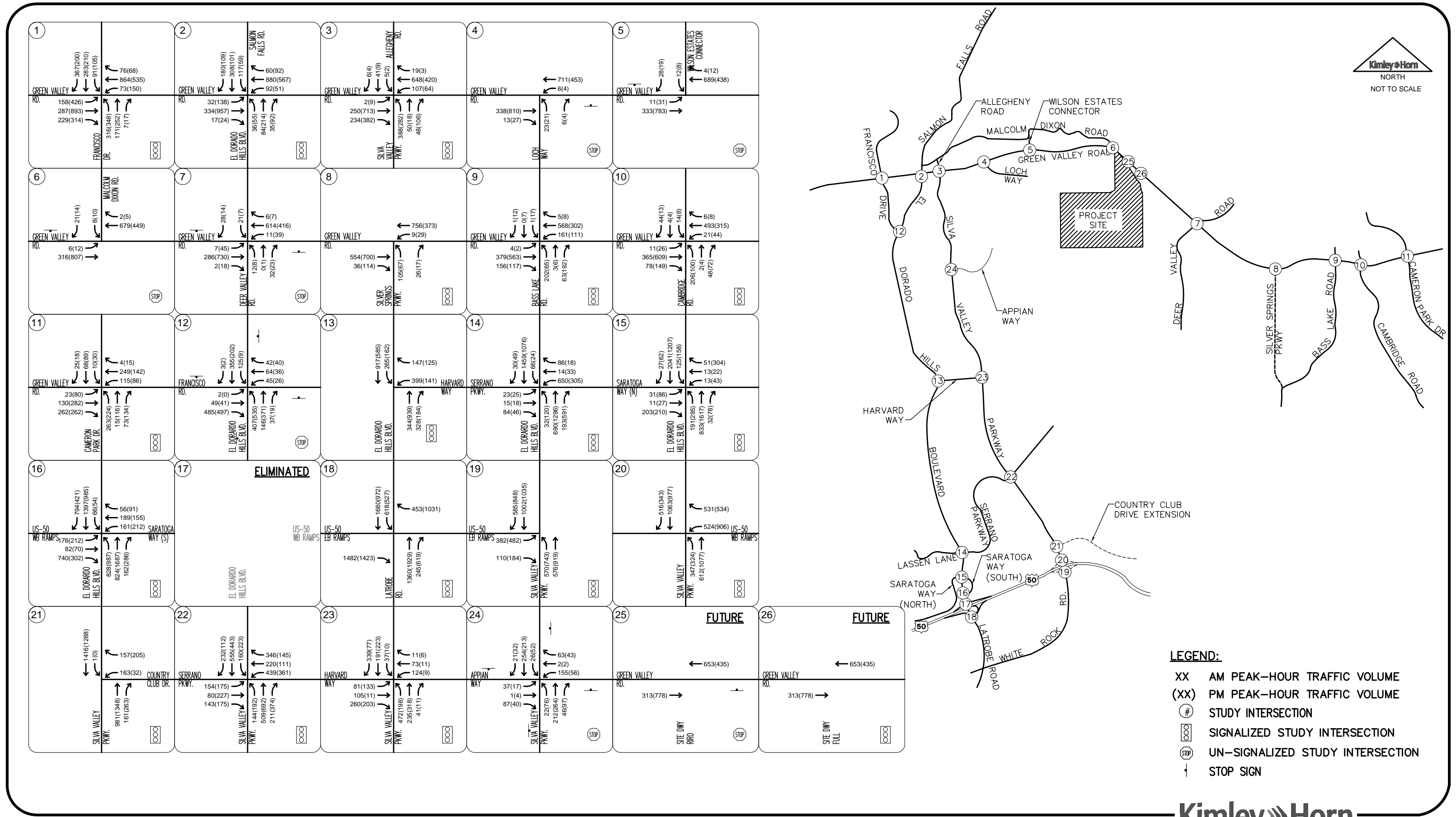


FIGURE 1
EXISTING PLUS APPROVED PROJECTS (2018) PEAK-HOUR VOLUMES

DIXON RANCH
EL DORADO HILLS, CA

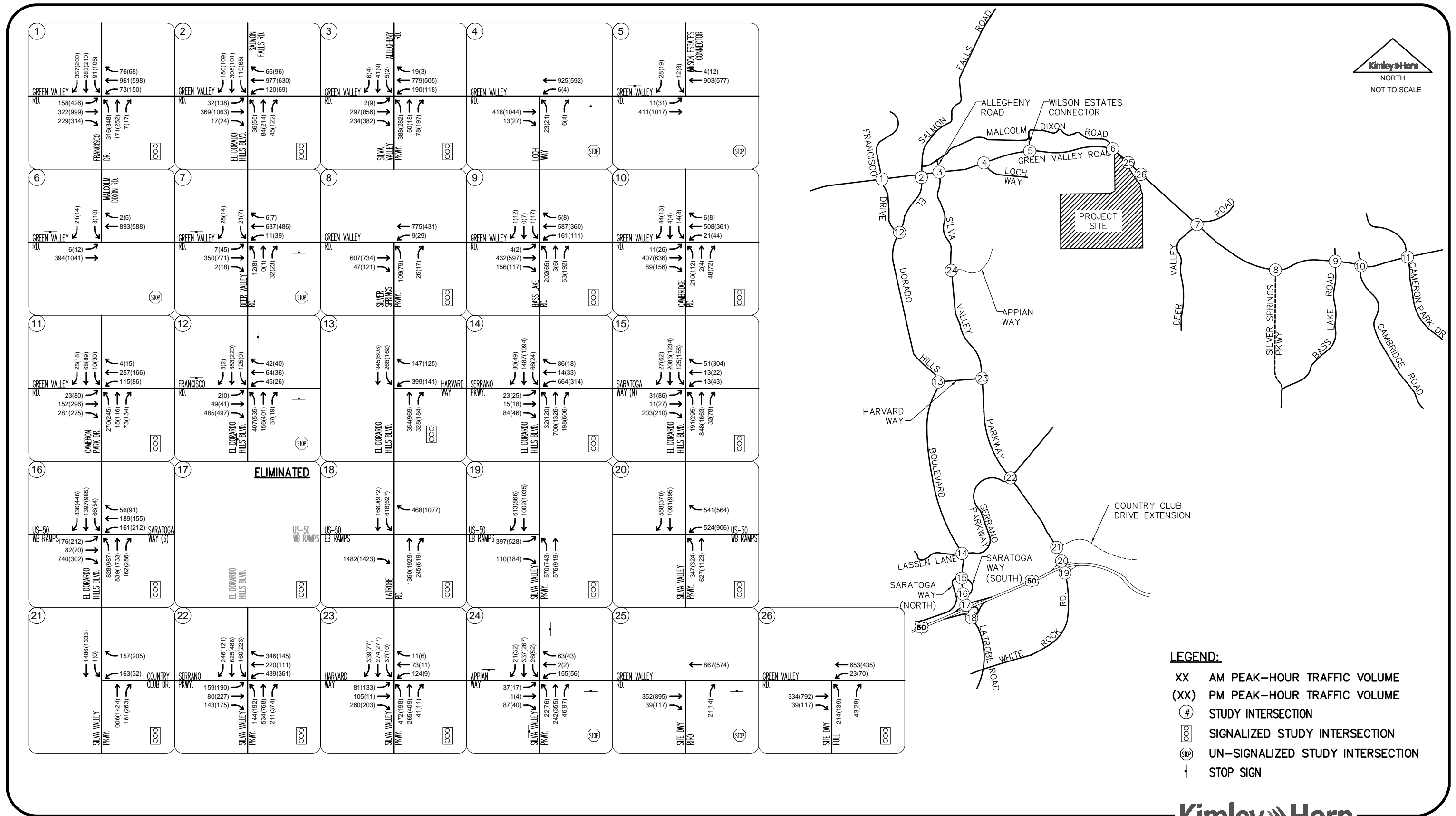
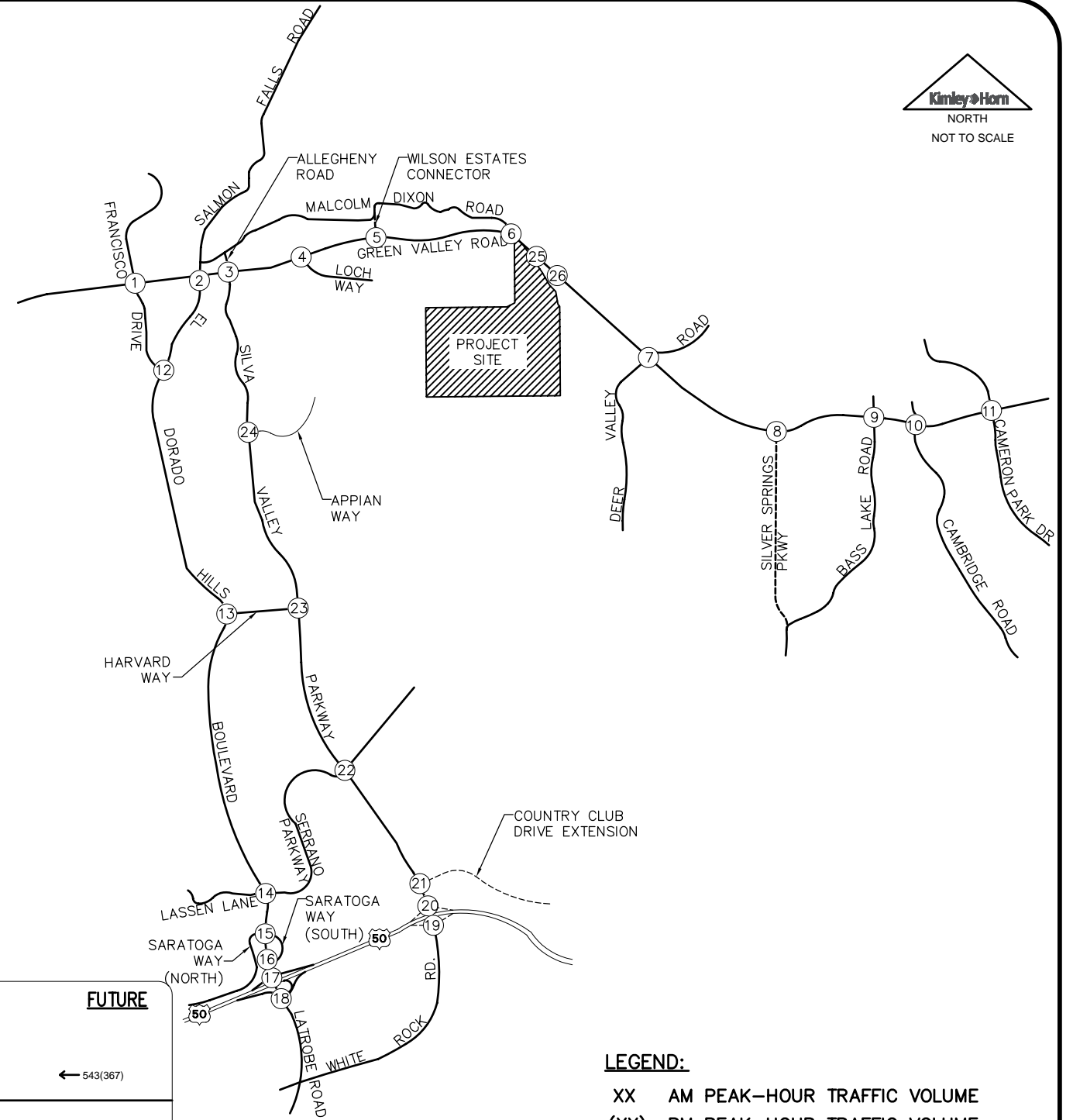
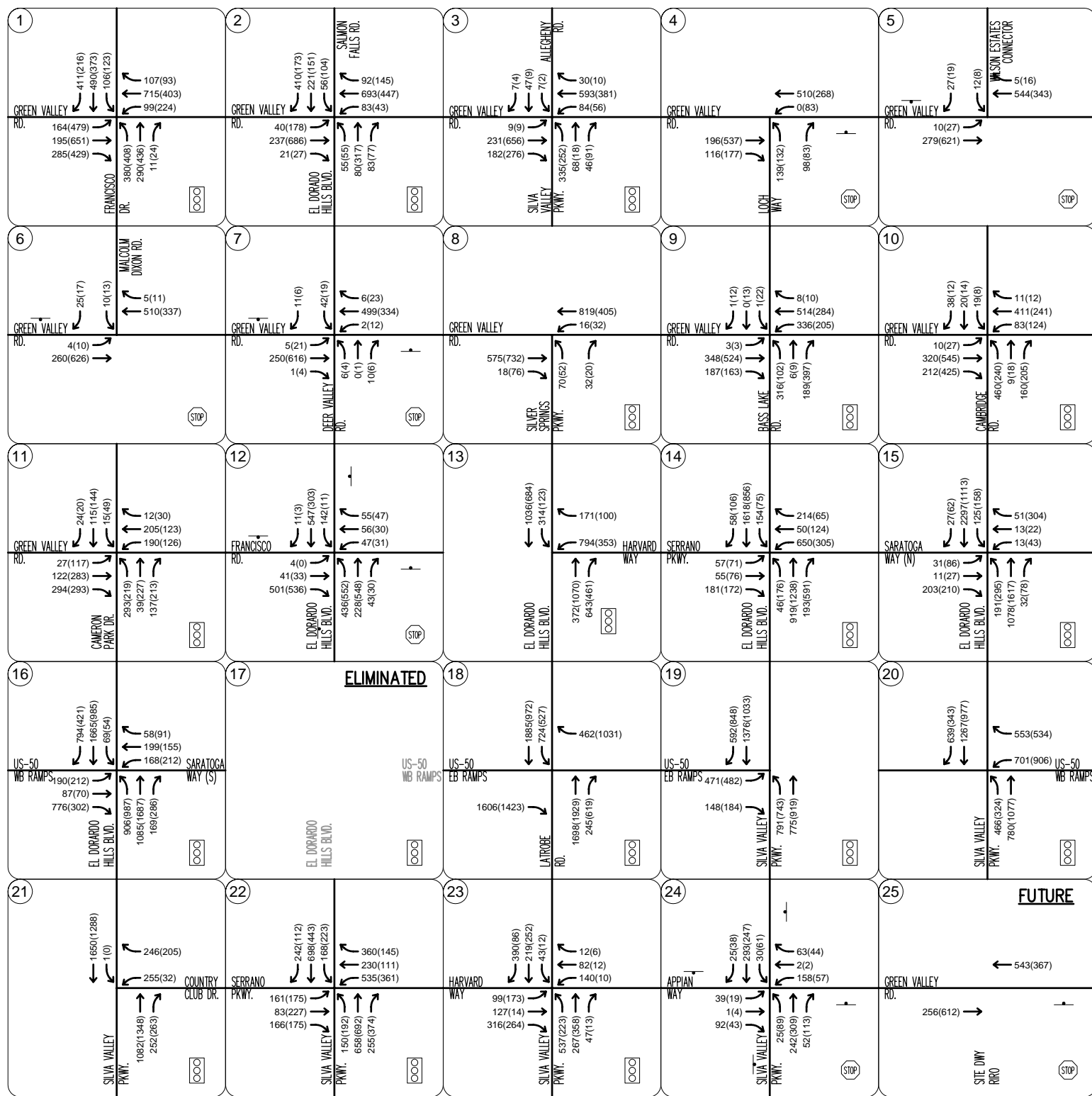


FIGURE 2
EXISTING PLUS APPROVED PROJECTS (2018) PLUS PROPOSED PROJECT PEAK-HOUR VOLUMES

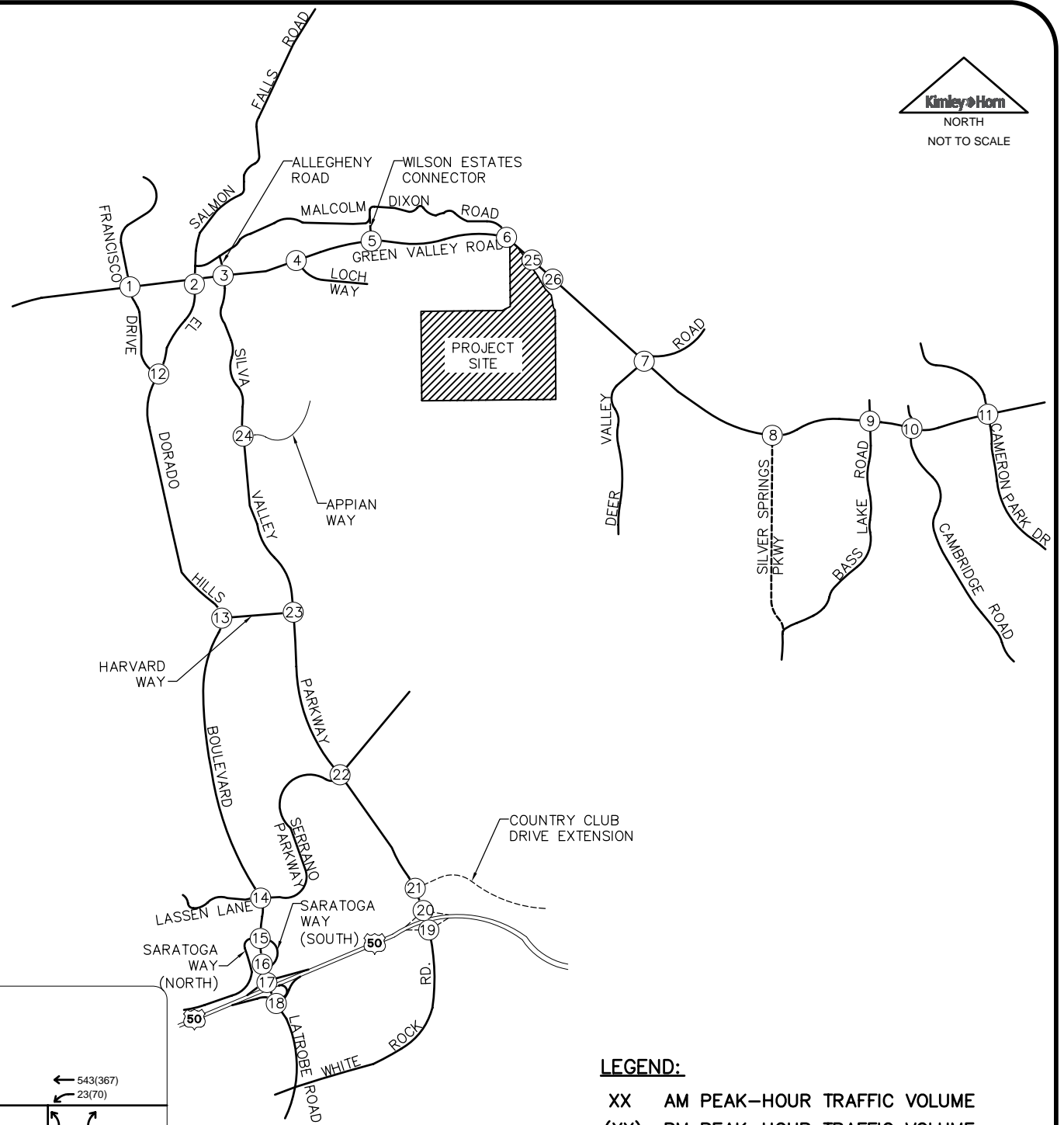
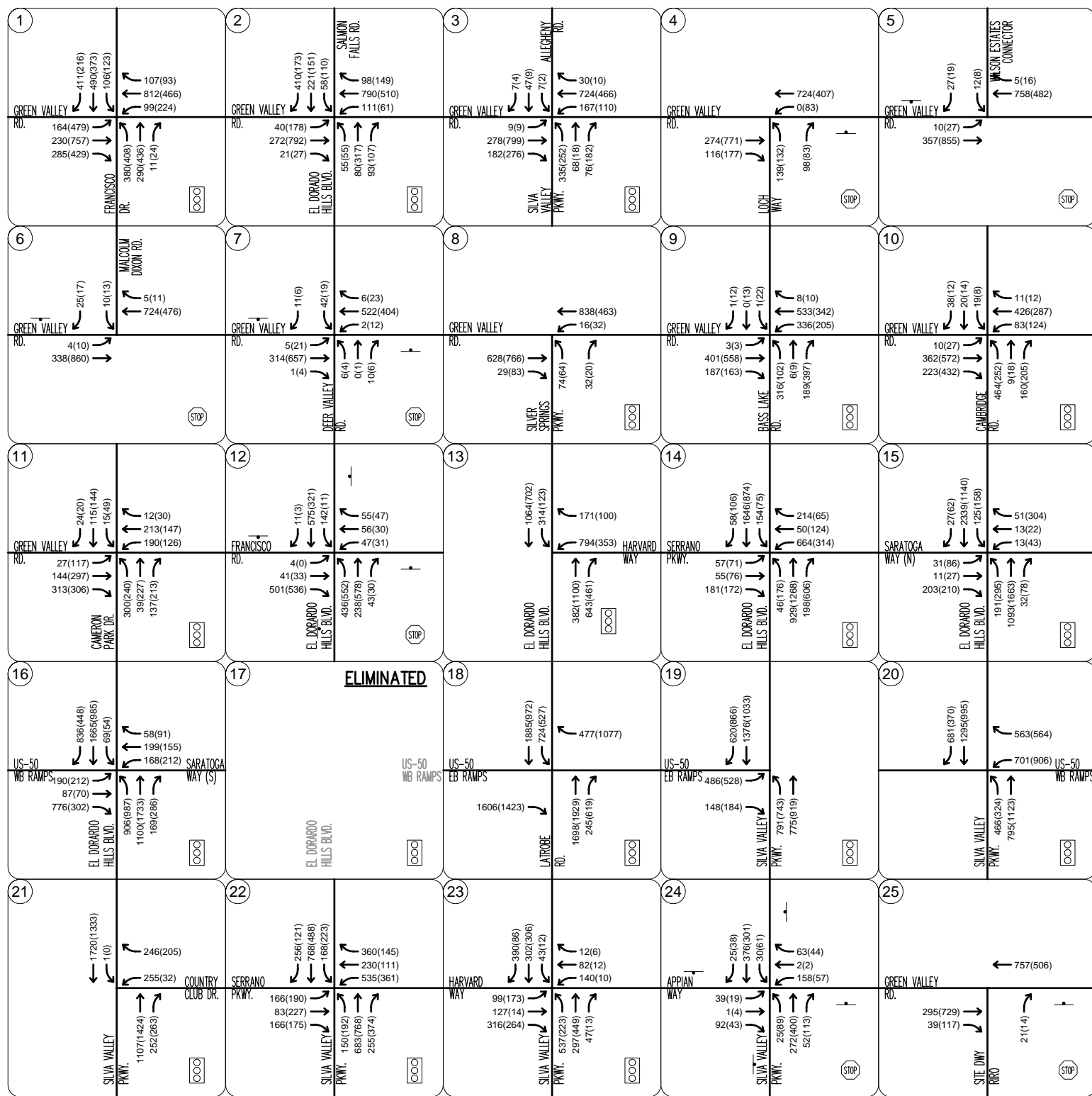
DIXON RANCH
EL DORADO HILLS, CA



- LEGEND:**
- XX AM PEAK-HOUR TRAFFIC VOLUME
 - (XX) PM PEAK-HOUR TRAFFIC VOLUME
 - # STUDY INTERSECTION
 - Signalized Symbol SIGNALIZED STUDY INTERSECTION
 - Un-signalized Symbol UN-SIGNALIZED STUDY INTERSECTION
 - ↑ STOP SIGN



FIGURE 3
CUMULATIVE (2035) PEAK-HOUR VOLUMES



- LEGEND:**
- XX AM PEAK-HOUR TRAFFIC VOLUME
 - (XX) PM PEAK-HOUR TRAFFIC VOLUME
 - # STUDY INTERSECTION
 - OOO SIGNALIZED STUDY INTERSECTION
 - STOP UN-SIGNALIZED STUDY INTERSECTION
 - ↑ STOP SIGN



FIGURE 4
CUMULATIVE (2035) PLUS PROPOSED PROJECT PEAK-HOUR VOLUMES

DIXON RANCH
EL DORADO HILLS, CA

Attachment A
Existing (2014) Freeway Analysis Worksheets

Basic

Segment Inputs				Existing Conditions														
				Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures							
	Length (ft)	Number of Lanes (N)	Interchange Density (I/mi)	AM Peak	PM Peak	V _p	FFS	S	D	LOS	V _p	FFS	S	D	LOS			
				(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)					
Westbou Eastbound	West of Latrobe Rd SB Off Ramp	6690	3	0.33	2,467	4,120	920.656	74.12	75	74.9303	12.287	B	1537.536	74.12	75	71.8014	21.4	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	1990	3	0.50	883	2,376	329.525	73.6	75	70.0236	4.7059	A	886.6957	73.6	75	74.8579	11.845	B
	East of Latrobe Rd On Ramp	7515	3	0.50	1,333	3,287	497.46	73.6	75	72.2043	6.8896	A	1226.67	73.6	75	74.4312	16.481	B
	East of El Dorado Hills Blvd Off Ramp	7270	2	0.50	3,511	2,060	1965.4	73.6	75	64.6829	30.385	D	1153.152	73.6	75	74.7403	15.429	B
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	3565	2	0.50	2,624	1,466	1468.87	73.6	75	72.5664	20.242	C	820.6413	73.6	75	74.6439	10.994	A
	West of El Dorado Hills Blvd On Ramp	5890	2	0.33	4,240	3,233	2373.48	74.12	75	54.1171	43.858	E	1809.777	74.12	75	67.741	26.716	D
Universal Inputs: PHF 0.92 (P ₁) 6% f _{hw} 0.970873786																		

Merge

Segment Inputs							AM Flow Inputs			AM LOS Performance Measures										Existing C
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/in)						(pc/mi/in)		
ω ∞ Latrobe Rd On Ramp	3	1	110	1333	883	450	1492	989	504	28	0.5806	573.95	7200	207	430	574	0.2073	12.96	B	
Σ ∞ El Dorado Hills Blvd On Ramp	2	1	795	4240	2624	1616	4747	2938	1809	84	1	2937.7	4800	0	2203	2938	0.9889	36.684	E	

Universal Inputs:

Length	1500	(ft)
S _{FF}	70	(mi/h)
S _{FR}	35	(mi/h)
PHF	0.92	
(P _c)	6%	
f _{HV}	0.970873786	

Merge

Segment Inputs			Conditions																
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	PM Flow Inputs			PM LOS Performance Measures												
				Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	V ₁₂	Capacity	V ₃	V _{12a}	v/c	D	LOS	
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/hr)	(pc/h)					(pc/h/in)					(pc/mi/in)
← ∞ Latrobe Rd On Ramp	3	1	110	3287	2376	911	3680	2660	1020	76	0.5806	1544.4	7200	558	1158	1544	0.5111	24.318	C
→ ∞ El Dorado Hills Blvd On Ramp	2	1	795	3233	1466	1767	3620	1641	1978	47	1	1641.3	4800	0	1231	1641	0.7541	27.813	C

Universal Inputs:

Length 1500 (ft)
S_{FF} 70 (mi/h)
S_{FR} 35 (mi/h)
PHF 0.92
(P_a) 6%
f_{REV} 0.970873786

Diverge

Segment Inputs				AM Flow Inputs													Existing C	
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume	Upstream Volume	Ramp Volume	v _D	v _F	v _R	P _{FD}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS	
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/in)	(pc/h/in)	(pc/h/in)		(pc/h/in)				(pc/mi/in)			
↳ Latrobe SB Off Ramp	3	1	2088	1191	2467	1276	344.826	2762	1428.6	0.6094	2241.1	7200	260	1681	2241	0.3836	22.266	C
↳ Latrobe NB Off Ramp	3	1	-	883	1191	308	-	1333.4	344.83	0.7108	1047.5	7200	286	786	1048	0.1852	12.001	B
↳ El Dorado Hills Blvd Off Ramp	2	1	-	2624	3511	887	-	3930.8	993.05	1	3930.8	4800	0	2948	3931	0.8189	36.347	E
Universal Inputs:																		
Leng	1500		(ft)															
S _{FF}	70		(mi/h)															
S _{FR}	35		(mi/h)															
PHF	0.92																	
P ₊	6%																	
I _{IV}	0.970873786																	

Diverge

Segment Inputs				Conditions															
				PM Flow Inputs			PM LOS Performance Measures												
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v _D	v _F	v _R	P _{FD}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
																		L _{EQ}	(veh/h)
	(N)		(ft)	(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/in)	(pc/h/in)	(pc/h/in)		(pc/h/in)							
↳ Latrobe SB Off Ramp	3	1	2088	140	3373	4120	747	1116.21	4612.6	836.32	0.6855	3424.9	7200	594	2569	3425	0.6406	32.446	D
↳ Latrobe NB Off Ramp	3	1	-	140	2376	3373	997	-	3776.3	1116.2	0.6142	2750.2	7200	1026	2063	2750	0.5245	26.643	C
↳ El Dorado Hills Blvd Off Ramp	2	1	-	190	1466	2060	594	-	2306.3	665.02	1	2306.3	4800	0	1730	2306	0.4805	22.376	C
Universal Inputs: Length 1500 (ft) S _{FF} 70 (mi/h) S _{FR} 35 (mi/h) PHF 0.92 P ₁ 6% P ₂ 0.970873786																			

Basic

Segment Inputs				Existing Plus Project Conditions														
				Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures							
	Length (ft)	Number of Lanes (N)	Interchange Density (I/mi)	PM		V _p (pc/h/ln)	FFS (mi/h)	S (mi/h)	D (pc/mi/ln)	LOS	V _p (pc/h/ln)	FFS (mi/h)	S (mi/h)	D (pc/mi/ln)	LOS			
				AM Peak (veh/h)	Peak (veh/h)													
Westbou. Eastbound	West of Latrobe Rd SB Off Ramp	6690	3	0.33	2,467	4,120	920.656	74.12	75	74.9303	12.287	B	1537.536	74.12	75	71.8014	21.4	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	1990	3	0.50	868	2,330	323.928	73.6	75	69.9402	4.6315	A	869.529	73.6	75	74.8116	11.623	B
	East of Latrobe Rd On Ramp	7515	3	0.50	1,318	3,241	491.862	73.6	75	72.1417	6.818	A	1209.504	73.6	75	74.5141	16.232	B
	East of El Dorado Hills Blvd Off Ramp	7270	2	0.50	3,511	2,060	1965.4	73.6	75	64.6829	30.385	D	1153.152	73.6	75	74.7403	15.429	B
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	3565	2	0.50	2,624	1,466	1468.87	73.6	75	72.5664	20.242	C	820.6413	73.6	75	74.6439	10.994	A
	West of El Dorado Hills Blvd On Ramp	5890	2	0.33	4,282	3,260	2396.99	74.12	75	53.396	44.891	E	1824.891	74.12	75	67.4675	27.048	D
Universal Inputs: PHF 0.92 (P ₁) 6% f _{hw} 0.970873786																		

Merge

Segment Inputs							AM Flow Inputs					AM LOS Performance Measures					Existing Plus Pr		
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS	
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/in)						(pc/mi/in)	
ω ∞ Latrobe Rd On Ramp	3	1	110	1318	868	450	1476	972	504	28	0.5806	564.2	7200	204	423	564	0.2049	12.884	B
Σ ∞ El Dorado Hills Blvd On Ramp	2	1	795	4282	2624	1658	4794	2938	1856	84	1	2937.7	4800	0	2203	2938	0.9987	37.03	E

Universal Inputs:

Length 1500 (ft)
 S_{FF} 70 (mi/h)
 S_{FR} 35 (mi/h)
 PHF 0.92
 P_c 6%
 f_{HV} 0.970873786

Merge

Subject Conditions															
PM Flow Inputs			PM LOS Performance Measures												
Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v_D	v_F	v_R	v_F/S_{FR}	P_{FM}	v_{12}	Capacity	v_3	v_{12a}	v/c	D	LOS	
(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/hr)	(pc/h)			(pc/h/ln)						(pc/mi/ln)	
3241	2330	911	3629	2609	1020	75	0.5806	1514.5	7200	547	1136	1514	0.504	24.085	C
3260	1466	1794	3650	1641	2009	47	1	1641.3	4800	0	1231	1641	0.7604	28.035	D

Diverge

										Existing Plus Proj									
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures												
	Number of Lanes	Number of Ramp Lanes	L _{EQ} (ft)	Length of Deceleration Lane (L _D) (ft)	Downstream Volume (veh/h)	Upstream Volume (veh/h)	Ramp Volume (veh/h)	V _D	V _F	V _R	P _{FD}	V ₁₂	Capacity	V ₃	V _{12a}	v/c	D	LOS	
								(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)			(pc/mi/ln)				
☒ Latrobe SB Off Ramp	3	1	2185	140	1191	2467	1276	361.62	2762	1428.6	0.6119	2244.5	7200	259	1683	2244	0.3836	22.294	C
☒ Latrobe NB Off Ramp	3	1	-	140	868	1191	323	-	1333.4	361.62	0.71	1051.6	7200	282	789	1052	0.1852	12.036	B
☒ El Dorado Hills Blvd Off Ramp	2	1	-	190	2624	3511	887	-	3930.8	993.05	1	3930.8	4800	0	2948	3931	0.8189	36.347	E
Universal Inputs: Leng 1500 (ft) S _{FF} 70 (mi/h) S _{TR} 35 (mi/h) PHF 0.92 P _(P-1) 6% I _{IV} 0.970873786																			

Diverge

PM Flow Inputs			PM LOS Performance Measures											
Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v_D	v_F	v_R	P_{FD}	v_{12}	Capacity	v_3	v_{12a}	v/c	D	LOS	
(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)					(pc/mi/ln)		
3373	4120	747	1167.71	4612.6	836.32	0.6932	3453.9	7200	579	2590	3454	0.6406	32.696	D
2330	3373	1043	-	3776.3	1167.7	0.6119	2763.8	7200	1012	2073	2764	0.5245	26.761	C
1466	2060	594	-	2306.3	665.02	1	2306.3	4800	0	1730	2306	0.4805	22.376	C

Attachment B

Existing plus Approved Projects (2018) Analysis Worksheets

Dixon Ranch
1: Green Valley Rd. & Francisco Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

	↖	→	↘	↙	←	↖	↙	↘	↙	↘	↙	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Volume (vph)	158	287	229	73	864	76	316	171	7	91	283	367
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.7	5.7	4.0	5.7	5.7	4.0	4.5		4.0	4.5	4.5
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.95		1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99		1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00
Satd. Flow (prot)	3433	3539	1583	1770	3539	1583	3433	3517		1770	1863	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00		0.95	1.00	1.00
Satd. Flow (perm)	3433	3539	1583	1770	3539	1583	3433	3517		1770	1863	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92		0.92	0.92	0.92
Adj. Flow (vph)	172	312	249	79	939	83	343	186		8	99	308
RTOR Reduction (vph)	0	0	164	0	0	56	0	4		0	0	127
Lane Group Flow (vph)	172	312	85	79	939	27	343	190		99	308	272
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA		Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						4
Actuated Green, G (s)	5.0	26.3	26.3	3.8	25.1	25.1	9.9	23.7		4.6	18.4	18.4
Effective Green, g (s)	5.0	26.3	26.3	3.8	25.1	25.1	9.9	23.7		4.6	18.4	18.4
Actuated g/C Ratio	0.07	0.34	0.34	0.05	0.33	0.33	0.13	0.31		0.06	0.24	0.24
Clearance Time (s)	4.0	5.7	5.7	4.0	5.7	5.7	4.0	4.5		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	224	1215	543	87	1159	518	443	1088		106	447	380
v/s Ratio Prot	c0.05	0.09		0.04	c0.27		c0.10	0.05		0.06	0.17	
v/s Ratio Perm			0.05			0.02						c0.17
w/c Ratio	0.77	0.26	0.16	0.91	0.81	0.05	0.77	0.17		0.93	0.69	0.72
Uniform Delay, d1	35.2	18.1	17.5	36.2	23.6	17.6	32.3	19.3		35.8	26.5	26.7
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	14.6	0.1	0.1	66.1	4.4	0.0	8.2	0.1		66.1	4.4	6.3
Delay (s)	49.8	18.2	17.6	102.3	28.0	17.7	40.5	19.4		102.0	30.9	33.0
Level of Service	D	B	B	F	C	B	D	B		F	C	C
Approach Delay (s)		25.4			32.5			32.9			40.7	
Approach LOS		C			C			C			D	
Intersection Summary												
HCM 2000 Control Delay			33.0									C
HCM 2000 Volume to Capacity ratio			0.77									
Actuated Cycle Length (s)			76.6									18.2
Intersection Capacity Utilization			67.5%									C
Analysis Period (min)			15									
c Critical Lane Group												

Dixon Ranch
2: El Dorado Hills Blvd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

	↖	→	↘	↙	←	↖	↙	↘	↙	↘	↙	↘
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Volume (vph)	32	334	17	92	880	60	36	84	35	117	308	180
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.5	6.0		3.5	6.0		4.0	4.0			5.5	5.5
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	1.00
Frt	1.00	0.99		1.00	0.99		1.00	0.96			1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00			0.99	1.00
Satd. Flow (prot)	1770	1850		1770	1845		1770	1780			1837	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00			0.99	1.00
Satd. Flow (perm)	1770	1850		1770	1845		1770	1780			1837	1583
Peak-hour factor, PHF	0.92	0.92		0.92	0.92		0.92	0.92			0.92	0.92
Adj. Flow (vph)	35	363		18	100		957	65	39	91	38	127
RTOR Reduction (vph)	0	1		0	2		0	0	11	0	0	115
Lane Group Flow (vph)	35	380		100	1020		39	118	0	0	462	81
Turn Type	Prot	NA		Prot	NA		Split	NA		Split	NA	Perm
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases												3
Actuated Green, G (s)	3.5	62.8		11.3	70.6		14.7	14.7			34.1	34.1
Effective Green, g (s)	3.5	62.8		11.3	70.6		14.7	14.7			34.1	34.1
Actuated g/C Ratio	0.02	0.44		0.08	0.50		0.10	0.10			0.24	0.24
Clearance Time (s)	3.5	6.0		3.5	6.0		4.0	4.0			5.5	5.5
Vehicle Extension (s)	2.5	5.0		2.5	5.0		4.5	4.5			4.5	4.5
Lane Grp Cap (vph)	43	818		140	917		183	184			441	380
v/s Ratio Prot	0.02	0.21		c0.06	c0.55		0.02	c0.07			c0.25	
v/s Ratio Perm												0.05
w/c Ratio	0.81	0.46		0.71	1.11		0.21	0.64			1.05	0.21
Uniform Delay, d1	68.9	27.8		63.7	35.7		58.3	61.1			53.9	43.2
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	1.00
Incremental Delay, d2	68.4	0.9		14.9	65.7		1.0	9.1			55.9	0.5
Delay (s)	137.2	28.6		78.6	101.4		59.3	70.2			109.8	43.7
Level of Service	F	C		E	F		E	E			F	D
Approach Delay (s)		37.8			99.4			67.7			90.1	
Approach LOS		D			F			E			F	
Intersection Summary												
HCM 2000 Control Delay					83.7							F
HCM 2000 Volume to Capacity ratio					1.03							
Actuated Cycle Length (s)					141.9						19.0	
Intersection Capacity Utilization					95.5%							F
Analysis Period (min)					15							
c Critical Lane Group												

Dixon Ranch
3: Silva Valley Pkwy. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↗	↖	↘	↙	↔	↕	↗	↖	↘	↙
Volume (vph)	2	250	234	107	648	19	388	50	48	5	41	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6			4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	0.93			0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)	1770	1863	1583	1770	1855		1770	1726			1824	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (perm)	1770	1863	1583	1770	1855		1770	1726			1824	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	2	272	254	116	704		21	422	54	5	45	7
RTOR Reduction (vph)	0	0	163	0	1	0	0	27	0	0	5	0
Lane Group Flow (vph)	2	272	91	116	724		0	422	79	0	52	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA		Split	NA	
Protected Phases	1	6		5	2		8	8		4	4	
Permitted Phases			6									
Actuated Green, G (s)	0.7	35.7	35.7	10.7	45.7		28.7	28.7			6.5	
Effective Green, g (s)	0.7	35.7	35.7	10.7	45.7		28.7	28.7			6.5	
Actuated g/C Ratio	0.01	0.36	0.36	0.11	0.46		0.29	0.29			0.07	
Clearance Time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6			4.0	
Vehicle Extension (s)	2.5	3.0	3.0	2.5	3.0		2.5	2.5			2.5	
Lane Grp Cap (vph)	12	665	565	189	848		508	495			118	
v/s Ratio Prot	0.00	0.15		c0.07	c0.39		c0.24	0.05			c0.03	
v/s Ratio Perm			0.06									
v/c Ratio	0.17	0.41	0.16	0.61	0.85		0.83	0.16			0.44	
Uniform Delay, d1	49.3	24.2	21.9	42.6	24.1		33.3	26.6			45.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2	4.7	0.4	0.1	5.0	8.4		10.9	0.1			1.9	
Delay (s)	54.1	24.6	22.0	47.6	32.5		44.2	26.7			46.9	
Level of Service	D	C	C	D	C		D	C			D	
Approach Delay (s)		23.5			34.6			40.7			46.9	
Approach LOS		C			C			D			D	
Intersection Summary												
HCM 2000 Control Delay			33.6									C
HCM 2000 Volume to Capacity ratio			0.82									
Actuated Cycle Length (s)			99.9					Sum of lost time (s)			18.3	
Intersection Capacity Utilization			78.7%					ICU Level of Service			D	
Analysis Period (min)			15									
c Critical Lane Group												

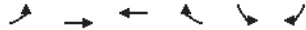
Dixon Ranch
4: Loch Way & Green Valley Rd

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↗	↖	↘	↔	↗
Volume (veh/h)	338	13	6	711	23	6
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	367	14	7	773	25	7
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			382		1160	374
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			382		1160	374
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		88	99
cM capacity (veh/h)			1177		215	672
Direction, Lane #						
	EB 1	WB 1	NB 1	NB 2		
Volume Total	382	779	25	7		
Volume Left	0	7	25	0		
Volume Right	14	0	0	7		
cSH	1700	1177	215	672		
Volume to Capacity	0.22	0.01	0.12	0.01		
Queue Length 95th (ft)	0	0	10	1		
Control Delay (s)	0.0	0.1	24.0	10.4		
Lane LOS		A	C	B		
Approach Delay (s)	0.0	0.1	21.1			
Approach LOS		C				
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			52.2%		ICU Level of Service	A
Analysis Period (min)			15			

Dixon Ranch
5: Green Valley Rd & Wilson Estates

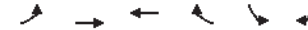
75-25 Modified Model, EPAP
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	11	333	689	4	12	28
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	362	749	4	13	30
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	753				1137	751
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	753				1137	751
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				94	93
cM capacity (veh/h)	857				220	411
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	374	753	43			
Volume Left	12	0	13			
Volume Right	0	4	30			
cSH	857	1700	326			
Volume to Capacity	0.01	0.44	0.13			
Queue Length 95th (ft)	1	0	11			
Control Delay (s)	0.5	0.0	17.7			
Lane LOS	A		C			
Approach Delay (s)	0.5	0.0	17.7			
Approach LOS			C			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization		46.5%		ICU Level of Service		A
Analysis Period (min)		15				

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd

75-25 Modified Model, EPAP
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	6	316	679	2	8	21
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	343	738	2	9	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	740				1096	739
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	740				1096	739
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				96	95
cM capacity (veh/h)	866				234	417
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	350	740	32			
Volume Left	7	0	9			
Volume Right	0	2	23			
cSH	866	1700	343			
Volume to Capacity	0.01	0.44	0.09			
Queue Length 95th (ft)	1	0	8			
Control Delay (s)	0.3	0.0	16.5			
Lane LOS	A		C			
Approach Delay (s)	0.3	0.0	16.5			
Approach LOS			C			
Intersection Summary						
Average Delay			0.5			
Intersection Capacity Utilization		45.9%		ICU Level of Service		A
Analysis Period (min)		15				

Dixon Ranch
7: Deer Valley Rd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↖	↗		↖	↗			↕			↕		
Volume (veh/h)	7	286	2	11	614	6	12	0	32	21	0	28	
Sign Control		Free			Free			Stop			Stop		
Grade		0%			0%			0%			0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	8	311	2	12	667	7	13	0	35	23	0	30	
Pedestrians													
Lane Width (ft)													
Walking Speed (ft/s)													
Percent Blockage													
Right turn flare (veh)													
Median type	None			None									
Median storage (veh)													
Upstream signal (ft)													
pX, platoon unblocked													
vC, conflicting volume	674			313			1049		1025		312		1055
vC1, stage 1 conf vol													
vC2, stage 2 conf vol													
vCu, unblocked vol	674			313			1049		1025		312		1055
tC, single (s)	4.1			4.1			7.1		6.5		7.1		6.5
tC, 2 stage (s)													
tF (s)	2.2			2.2			3.5		4.0		3.3		3.5
p0 queue free %	99			99			93		100		95		88
cM capacity (veh/h)	917			1247			189		231		728		191
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1							
Volume Total	8	313	12	674	48	53							
Volume Left	8	0	12	0	13	23							
Volume Right	0	2	0	7	35	30							
cSH	917	1700	1247	1700	410	286							
Volume to Capacity	0.01	0.18	0.01	0.40	0.12	0.19							
Queue Length 95th (ft)	1	0	1	0	10	17							
Control Delay (s)	9.0	0.0	7.9	0.0	14.9	20.4							
Lane LOS	A		A		B	C							
Approach Delay (s)	0.2		0.1		14.9		20.4						
Approach LOS					B		C						
Intersection Summary													
Average Delay	1.8												
Intersection Capacity Utilization	44.0%			ICU Level of Service			A						
Analysis Period (min)	15												

Dixon Ranch
8: Silver Springs Pkwy & Green Valley Rd

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↗	↘	↖	↗	↖	↘
Volume (vph)	554	36	9	756	105	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0		4.0		4.0	
Lane Util. Factor	1.00		1.00		1.00	
Frt	0.99		1.00		0.97	
Flt Protected	1.00		1.00		0.96	
Satd. Flow (prot)	1847		1862		1743	
Flt Permitted	1.00		0.99		0.96	
Satd. Flow (perm)	1847		1850		1743	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	602	39	10	822	114	28
RTOR Reduction (vph)	4	0	0	0	16	0
Lane Group Flow (vph)	637	0	0	832	126	0
Turn Type	NA		Perm		NA	
Protected Phases	4		8		2	
Permitted Phases	8					
Actuated Green, G (s)	24.4		24.4		8.8	
Effective Green, g (s)	24.4		24.4		8.8	
Actuated g/C Ratio	0.59		0.59		0.21	
Clearance Time (s)	4.0		4.0		4.0	
Vehicle Extension (s)	3.0		3.0		3.0	
Lane Grp Cap (vph)	1093		1095		372	
v/s Ratio Prot	0.34				c0.07	
v/s Ratio Perm			c0.45			
w/c Ratio	0.58		0.76		0.34	
Uniform Delay, d1	5.2		6.2		13.7	
Progression Factor	1.00		1.00		1.00	
Incremental Delay, d2	0.8		3.1		0.5	
Delay (s)	6.0		9.3		14.3	
Level of Service	A		A		B	
Approach Delay (s)	6.0		9.3		14.3	
Approach LOS	A		A		B	
Intersection Summary						
HCM 2000 Control Delay	8.4		HCM 2000 Level of Service		A	
HCM 2000 Volume to Capacity ratio	0.65					
Actuated Cycle Length (s)	41.2		Sum of lost time (s)		8.0	
Intersection Capacity Utilization	61.0%		ICU Level of Service		B	
Analysis Period (min)	15					
c Critical Lane Group						

Dixon Ranch
9: Bass Lake Rd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔	↔	↔	↔	↔
Volume (vph)	4	379	156	161	568	5	202	3	63	1	0	1
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0			4.0	4.0			4.0
Lane Util. Factor	1.00	1.00		1.00	1.00			1.00	1.00			1.00
Frt	1.00	0.96		1.00	1.00			1.00	0.85			0.93
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00			0.98
Satd. Flow (prot)	1770	1781		1770	1860			1775	1583			1695
Flt Permitted	0.95	1.00		0.95	1.00			0.95	1.00			0.98
Satd. Flow (perm)	1770	1781		1770	1860			1775	1583			1695
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	4	412	170	175	617	5	220	3	68	1	0	1
RTOR Reduction (vph)	0	14	0	0	0	0	0	0	55	0	2	0
Lane Group Flow (vph)	4	568	0	175	622	0	0	223	13	0	0	0
Turn Type	Prot	NA		Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases								2				
Actuated Green, G (s)	0.7	31.0		10.2	40.5		13.1	13.1		0.9		
Effective Green, g (s)	0.7	31.0		10.2	40.5		13.1	13.1		0.9		
Actuated g/C Ratio	0.01	0.44		0.14	0.57		0.18	0.18		0.01		
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
Lane Grp Cap (vph)	17	775		253	1058		326	291		21		
v/s Ratio Prot	0.00	c0.32		c0.10	0.33		c0.13			c0.00		
v/s Ratio Perm								0.01				
w/c Ratio	0.24	0.73		0.69	0.59		0.68	0.04		0.00		
Uniform Delay, d1	35.0	16.7		29.0	9.9		27.1	23.9		34.7		
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00		
Incremental Delay, d2	7.0	3.6		7.9	0.8		5.8	0.1		0.0		
Delay (s)	42.0	20.3		36.9	10.8		33.0	24.0		34.7		
Level of Service	D	C		D	B		C	C		C		
Approach Delay (s)		20.4			16.5			30.8				34.7
Approach LOS		C			B			C				C
Intersection Summary												
HCM 2000 Control Delay		20.4										C
HCM 2000 Volume to Capacity ratio		0.70										
Actuated Cycle Length (s)		71.2			Sum of lost time (s)			16.0				
Intersection Capacity Utilization		66.4%			ICU Level of Service							C
Analysis Period (min)		15										
c Critical Lane Group												

Dixon Ranch
10: Cambridge Rd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔	↔	↔	↔	↔
Volume (vph)	11	365	78	21	493	6	206	2	48	14	4	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0			4.0	4.0			4.0
Lane Util. Factor	1.00	1.00		1.00	1.00			1.00	1.00			1.00
Frt	1.00	0.97		1.00	1.00			1.00	0.86			0.90
Flt Protected	0.95	1.00		0.95	1.00			0.95	1.00			0.99
Satd. Flow (prot)	1770	1813		1770	1859			1770	1594			1664
Flt Permitted	0.95	1.00		0.95	1.00			0.95	1.00			0.99
Satd. Flow (perm)	1770	1813		1770	1859			1770	1594			1664
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	12	397	85	23	536	7	224	2	52	15	4	48
RTOR Reduction (vph)	0	9	0	0	1	0	0	41	0	0	45	0
Lane Group Flow (vph)	12	473	0	23	542	0	224	13	0	0	22	0
Turn Type	Prot	NA		Prot	NA		Split	NA		Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases												
Actuated Green, G (s)	0.6	22.2		1.2	22.8		11.9	11.9		3.7		
Effective Green, g (s)	0.6	22.2		1.2	22.8		11.9	11.9		3.7		
Actuated g/C Ratio	0.01	0.40		0.02	0.41		0.22	0.22		0.07		
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0		
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0		
Lane Grp Cap (vph)	19	731		38	770		382	344		111		
v/s Ratio Prot	0.01	0.26		c0.01	c0.29		c0.13	0.01		c0.01		
v/s Ratio Perm												
w/c Ratio	0.63	0.65		0.61	0.70		0.59	0.04		0.20		
Uniform Delay, d1	27.1	13.2		26.7	13.3		19.3	17.0		24.3		
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00		
Incremental Delay, d2	52.7	2.0		24.3	2.9		2.3	0.0		0.9		
Delay (s)	79.8	15.2		50.9	16.3		21.6	17.1		25.1		
Level of Service	E	B		D	B		C	B		C		
Approach Delay (s)		16.8			17.7			20.8				25.1
Approach LOS		B			B			C				C
Intersection Summary												
HCM 2000 Control Delay		18.3										B
HCM 2000 Volume to Capacity ratio		0.63										
Actuated Cycle Length (s)		55.0			Sum of lost time (s)			16.0				
Intersection Capacity Utilization		51.1%			ICU Level of Service							A
Analysis Period (min)		15										
c Critical Lane Group												

Dixon Ranch
11: Cameron Park Dr. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	23	130	262	115	249	4	263	15	73	10	68	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	1676		1770	1859		1770	1630		1770	1788	
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1770	1676		1770	1859		1770	1630		1770	1788	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	141	285	125	271	4	286	16	79	11	74	27
RTOR Reduction (vph)	0	105	0	0	1	0	0	53	0	0	23	0
Lane Group Flow (vph)	25	321	0	125	274	0	286	42	0	11	78	0
Turn Type	Prot	NA		Prot	NA		Prot	NA		Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases												
Actuated Green, G (s)	0.7	17.2		5.2	21.7		11.8	19.7		0.7	8.6	
Effective Green, g (s)	0.7	17.2		5.2	21.7		11.8	19.7		0.7	8.6	
Actuated g/C Ratio	0.01	0.29		0.09	0.37		0.20	0.34		0.01	0.15	
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	21	490		156	686		355	546		21	261	
v/s Ratio Prot	0.01	c0.19		c0.07	0.15		c0.16	0.03		0.01	c0.04	
v/s Ratio Perm												
v/c Ratio	1.19	0.65		0.80	0.40		0.81	0.08		0.52	0.30	
Uniform Delay, d1	29.0	18.2		26.3	13.7		22.4	13.3		28.9	22.4	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	261.4	3.1		24.8	0.4		12.5	0.1		21.6	0.6	
Delay (s)	290.4	21.3		51.1	14.1		34.9	13.4		50.5	23.1	
Level of Service	F	C		D	B		C	B		D	C	
Approach Delay (s)		36.3			25.7			29.5			25.7	
Approach LOS		D			C			C			C	
Intersection Summary												
HCM 2000 Control Delay			30.3									C
HCM 2000 Volume to Capacity ratio			0.64									
Actuated Cycle Length (s)			58.8						16.0			
Intersection Capacity Utilization			60.5%									B
Analysis Period (min)			15									
c Critical Lane Group												

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Sign Control		Stop			Stop			Stop		Stop		Stop
Volume (vph)	2	49	485	45	64	42	407	146	37	125	355	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	53	527	49	70	46	442	159	40	136	386	3
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	55	527	164	442	199	136	389					
Volume Left (vph)	2	0	49	442	0	136	0					
Volume Right (vph)	0	527	46	0	40	0	3					
Hadj (s)	0.04	-0.57	-0.07	0.53	-0.11	0.53	0.03					
Departure Headway (s)	7.2	3.2	6.6	6.6	5.9	6.7	6.2					
Degree Utilization, x	0.11	0.47	0.30	0.81	0.33	0.25	0.67					
Capacity (veh/h)	452	1116	507	537	591	515	557					
Control Delay (s)	11.0	9.0	12.5	30.4	10.6	10.8	19.9					
Approach Delay (s)	9.2		12.5	24.3		17.6						
Approach LOS	A		B	C		C						
Intersection Summary												
Delay							16.8					
Level of Service							C					
Intersection Capacity Utilization			67.3%			ICU Level of Service					C	
Analysis Period (min)			15									

Dixon Ranch
13: El Dorado Hills Blvd. & Harvard Way

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↗	↕	↕	↗	↘
Volume (vph)	399	147	344	328	265	917
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		0.97	0.95
Frt	1.00	0.85	0.93		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1770	1583	3280		3433	3539
Flt Permitted	0.95	1.00	1.00		0.95	1.00
Satd. Flow (perm)	1770	1583	3280		3433	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	434	160	374	357	288	997
RTOR Reduction (vph)	0	108	263	0	0	0
Lane Group Flow (vph)	434	52	468	0	288	997
Turn Type	NA	Perm	NA		Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8				
Actuated Green, G (s)	14.2	14.2	11.6		6.1	21.7
Effective Green, g (s)	14.2	14.2	11.6		6.1	21.7
Actuated g/C Ratio	0.32	0.32	0.26		0.14	0.49
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	572	512	866		477	1749
v/s Ratio Prot	c0.25		0.14		0.08	c0.28
v/s Ratio Perm		0.03				
w/c Ratio	0.76	0.10	0.54		0.60	0.57
Uniform Delay, d1	13.3	10.4	13.9		17.8	7.8
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	5.7	0.1	0.7		2.2	0.5
Delay (s)	19.0	10.5	14.6		19.9	8.3
Level of Service	B	B	B		B	A
Approach Delay (s)	16.7		14.6			10.9
Approach LOS	B		B			B
Intersection Summary						
HCM 2000 Control Delay			13.2		HCM 2000 Level of Service	B
HCM 2000 Volume to Capacity ratio			0.73			
Actuated Cycle Length (s)			43.9		Sum of lost time (s)	12.0
Intersection Capacity Utilization		59.7%			ICU Level of Service	B
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch
14: El Dorado Hills Blvd. & Serrano Pkwy.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↗	↕	↘	↗	↕	↘	↗	↕	↘	↗	↕
Volume (vph)	23	15	84	650	14	86	32	690	193	66	1459	30
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.0	3.0		3.0	3.0		3.0	5.2	4.0	3.0	5.2	
Lane Util. Factor	1.00	1.00		0.95	0.95		1.00	0.95	1.00	1.00	0.95	
Frt	1.00	0.87		1.00	0.97		1.00	1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00		0.95	0.96		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1625		1681	1648		1770	3539	1583	1770	3528	
Flt Permitted	0.95	1.00		0.95	0.96		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1625		1681	1648		1770	3539	1583	1770	3528	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	16	91	707	15	93	35	750	210	72	1586	33
RTOR Reduction (vph)	0	31	0	0	8	0	0	0	0	0	1	0
Lane Group Flow (vph)	25	76	0	410	397	0	35	750	210	72	1618	0
Turn Type	Split	NA		Split	NA		Prot	NA	Free	Prot	NA	
Protected Phases	7	7		8	8		5	2		1	6	
Permitted Phases									Free			
Actuated Green, G (s)	6.7	6.7		33.3	33.3		3.6	68.9	130.0	6.9	72.2	
Effective Green, g (s)	6.7	6.7		33.3	33.3		3.6	68.9	130.0	6.9	72.2	
Actuated g/C Ratio	0.05	0.05		0.26	0.26		0.03	0.53	1.00	0.05	0.56	
Clearance Time (s)	3.0	3.0		3.0	3.0		3.0	5.2		3.0	5.2	
Vehicle Extension (s)	0.2	0.2		0.2	0.2		0.2	0.2		0.2	0.2	
Lane Grp Cap (vph)	91	83		430	422		49	1875	1583	93	1959	
v/s Ratio Prot	0.01	c0.05		c0.24	0.24		c0.02	0.21		0.04	c0.46	
v/s Ratio Perm									0.13			
w/c Ratio	0.27	0.91		0.95	0.94		0.71	0.40	0.13	0.77	0.83	
Uniform Delay, d1	59.3	61.4		47.6	47.4		62.7	18.2	0.0	60.8	23.7	
Progression Factor	1.00	1.00		1.00	1.00		0.76	0.60	1.00	1.00	1.00	
Incremental Delay, d2	0.6	68.8		31.3	28.9		32.9	0.6	0.2	29.8	4.1	
Delay (s)	59.9	130.2		78.9	76.3		80.4	11.5	0.2	90.5	27.9	
Level of Service	E	F		E	E		F	B	A	F	C	
Approach Delay (s)		116.9			77.6			11.6			30.6	
Approach LOS		F			E			B			C	
Intersection Summary												
HCM 2000 Control Delay				39.0				HCM 2000 Level of Service		D		
HCM 2000 Volume to Capacity ratio				0.86								
Actuated Cycle Length (s)				130.0				Sum of lost time (s)		14.2		
Intersection Capacity Utilization				83.3%				ICU Level of Service		E		
Analysis Period (min)				15								
c Critical Lane Group												

Dixon Ranch
15: El Dorado Hills Blvd. & Saratoga Wy. (North)

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Volume (vph)	31	11	203	13	13	51	191	833	32	125	2041	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	1.00	1.00	1.00	0.91	0.99	1.00	0.95	0.95	1.00
Flt Protected	0.95	0.98	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1681	1728	1583	1770	1640	1770	5057	1770	3532	1770	3532	1900
Flt Permitted	0.95	0.98	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1681	1728	1583	1770	1640	1770	5057	1770	3532	1770	3532	1900
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	34	12	221	14	14	55	208	905	35	136	2218	29
RTOR Reduction (vph)	0	0	209	0	53	0	0	2	0	0	0	0
Lane Group Flow (vph)	23	23	12	14	16	0	208	938	0	136	2247	0
Turn Type	Split	NA	Prot	Split	NA	Prot	NA	Prot	NA	Prot	NA	Prot
Protected Phases	7	7	7	8	8		1	6		5	2	
Permitted Phases												
Actuated Green, G (s)	6.8	6.8	6.8	4.8	4.8		10.0	82.7		18.0	90.7	
Effective Green, g (s)	6.8	6.8	6.8	4.8	4.8		10.0	84.4		18.0	92.4	
Actuated g/C Ratio	0.05	0.05	0.05	0.04	0.04		0.08	0.65		0.14	0.71	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	5.7		4.0	5.7	
Vehicle Extension (s)	0.2	0.2	0.2	0.2	0.2		0.2	4.2		0.2	4.2	
Lane Grp Cap (vph)	87	90	82	65	60		136	3283		245	2510	
v/s Ratio Prot	c0.01	0.01	0.01	0.01	c0.01		c0.12	0.19		0.08	c0.64	
v/s Ratio Perm												
w/c Ratio	0.26	0.26	0.14	0.22	0.27		1.53	0.29		0.56	0.90	
Uniform Delay, d1	59.2	59.2	58.8	60.8	60.9		60.0	9.8		52.3	14.9	
Progression Factor	1.00	1.00	1.00	1.00	1.00		0.85	0.54		0.85	0.45	
Incremental Delay, d2	0.6	0.5	0.3	0.6	0.9		269.3	0.2		0.8	3.0	
Delay (s)	59.8	59.7	59.1	61.4	61.8		320.4	5.5		45.4	9.7	
Level of Service	E	E	E	E	E		F	A		D	A	
Approach Delay (s)		59.2			61.7			62.6			11.8	
Approach LOS		E			E			E			B	
Intersection Summary												
HCM 2000 Control Delay			31.1				HCM 2000 Level of Service				C	
HCM 2000 Volume to Capacity ratio			0.89									
Actuated Cycle Length (s)			130.0				Sum of lost time (s)			16.0		
Intersection Capacity Utilization			85.7%				ICU Level of Service			E		
Analysis Period (min)			15									
c Critical Lane Group												

Dixon Ranch
16: El Dorado Hills Blvd. & Saratoga Wy. (South)

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖	↖
Volume (vph)	176	82	740	161	189	56	828	824	162	66	1397	794
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	0.95	0.95	1.00	0.95	0.95	1.00	0.97	0.91	1.00	0.91	1.00	1.00
Flt Protected	0.95	0.98	1.00	0.95	1.00	1.00	0.95	1.00	0.98	1.00	0.95	1.00
Satd. Flow (prot)	1681	1737	1583	1681	1762	1583	3433	4960	1770	5085	1583	1900
Flt Permitted	0.95	0.98	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00
Satd. Flow (perm)	1681	1737	1583	1681	1762	1583	3433	4960	1770	5085	1583	1900
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	191	89	804	175	205	61	900	896	176	72	1518	863
RTOR Reduction (vph)	0	0	0	0	0	51	0	20	0	0	0	0
Lane Group Flow (vph)	138	142	804	157	223	10	900	1052	0	72	1518	863
Turn Type	Split	NA	Free	Split	NA	Perm	Prot	NA	Prot	NA	Free	Free
Protected Phases	4	4		8	8		5	2		1	6	
Permitted Phases			Free			8						Free
Actuated Green, G (s)	14.4	14.4	130.0	20.6	20.6	20.6	38.1	69.6	9.4	40.9	130.0	
Effective Green, g (s)	14.4	14.4	130.0	20.6	20.6	20.6	38.1	69.6	9.4	40.9	130.0	
Actuated g/C Ratio	0.11	0.11	1.00	0.16	0.16	0.16	0.29	0.54	0.07	0.31	1.00	
Clearance Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0		
Vehicle Extension (s)	3.0	3.0		2.2	2.2	2.2	3.0	3.5	2.0	2.5		
Lane Grp Cap (vph)	186	192	1583	266	279	250	1006	2655	127	1599	1583	
v/s Ratio Prot	c0.08	0.08		0.09	c0.13		c0.26	0.21	0.04	c0.30		
v/s Ratio Perm			0.51			0.01						0.55
w/c Ratio	0.74	0.74	0.51	0.59	0.80	0.04	0.89	0.40	0.57	0.95	0.55	
Uniform Delay, d1	56.0	56.0	0.0	50.8	52.7	46.3	44.0	17.8	58.3	43.5	0.0	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	0.88	0.95	0.72	0.67	1.00	
Incremental Delay, d2	14.7	13.9	1.2	2.6	14.0	0.0	8.6	0.4	1.7	7.8	0.7	
Delay (s)	70.7	69.8	1.2	53.3	66.7	46.3	47.5	17.3	43.4	36.9	0.7	
Level of Service	E	E	A	D	E	D	D	B	D	D	A	
Approach Delay (s)		19.0			59.1			31.1			24.4	
Approach LOS		B			E			C			C	
Intersection Summary												
HCM 2000 Control Delay			28.2				HCM 2000 Level of Service				C	
HCM 2000 Volume to Capacity ratio			0.88									
Actuated Cycle Length (s)			130.0				Sum of lost time (s)			16.0		
Intersection Capacity Utilization			80.9%				ICU Level of Service			D		
Analysis Period (min)			15									
c Critical Lane Group												

Dixon Ranch
18: Latrobe Rd. & US-50 EB Ramp

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations			↑↑			↑		↑↑↑	↑	↑	↑↑↑	↑
Volume (vph)	0	0	1482	0	0	453	0	1360	245	618	1680	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)			4.0			4.0		4.0	4.0	4.0	4.0	
Lane Util. Factor			0.88			1.00		0.91	1.00	1.00	0.86	
Frt			0.85			0.86		1.00	0.85	1.00	1.00	
Flt Protected			1.00			1.00		1.00	1.00	0.95	1.00	
Satd. Flow (prot)			2787			1611		5085	1583	1770	6408	
Flt Permitted			1.00			1.00		1.00	1.00	0.95	1.00	
Satd. Flow (perm)			2787			1611		5085	1583	1770	6408	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	1611	0	0	492	0	1478	266	672	1826	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	110	0	0	0
Lane Group Flow (vph)	0	0	1611	0	0	492	0	1478	156	672	1826	0
Turn Type		custom				Free	NA	Perm	Prot	NA		
Protected Phases		5					2		1	6		
Permitted Phases		1			Free			2				
Actuated Green, G (s)		50.2				65.0	27.8	27.8	29.2	36.0		
Effective Green, g (s)		50.2				65.0	27.8	27.8	29.2	36.0		
Actuated g/C Ratio		0.77				1.00	0.43	0.43	0.45	0.55		
Clearance Time (s)		4.0					4.0	4.0	4.0	4.0		
Vehicle Extension (s)		3.0					3.0	3.0	3.0	3.0		
Lane Grp Cap (vph)		2323				1611	2174	677	795	3549		
w/s Ratio Prot		0.22					c0.29		c0.38	0.28		
w/s Ratio Perm		0.35				0.31		0.10				
w/c Ratio		0.69				0.31	0.68	0.23	0.85	0.51		
Uniform Delay, d1		3.6				0.0	15.0	11.8	15.9	9.0		
Progression Factor		1.00				1.00	1.00	1.00	1.09	0.91		
Incremental Delay, d2		0.9				0.5	1.7	0.8	5.3	0.3		
Delay (s)		4.5				0.5	16.7	12.6	22.6	8.5		
Level of Service		A				A	B	B	C	A		
Approach Delay (s)		4.5			0.5		16.1			12.3		
Approach LOS		A			A		B			B		
Intersection Summary												
HCM 2000 Control Delay		10.5								B		
HCM 2000 Volume to Capacity ratio		0.76										
Actuated Cycle Length (s)		65.0								8.0		
Intersection Capacity Utilization		82.9%								E		
Analysis Period (min)		15										
c Critical Lane Group												

Dixon Ranch
19: Silva Valley Pkwy & EB US-50 Ramps

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑↓	↑				↑↑	↑↑		↑↑	↑↑	↑
Volume (vph)	382	0	110	0	0	0	570	576	0	0	1002	585
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0				4.0	4.0			4.0	4.0
Lane Util. Factor	0.95	0.91	0.95				0.97	0.95			0.95	1.00
Frt	1.00	0.99	0.85				1.00	1.00			1.00	0.85
Flt Protected	0.95	0.95	1.00				0.95	1.00			1.00	1.00
Satd. Flow (prot)	1681	1605	1504				3433	3539			3539	1583
Flt Permitted	0.95	0.95	1.00				0.95	1.00			1.00	1.00
Satd. Flow (perm)	1681	1605	1504				3433	3539			3539	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	415	0	120	0	0	0	620	626	0	0	1089	636
RTOR Reduction (vph)	0	56	89	0	0	0	0	0	0	0	0	292
Lane Group Flow (vph)	216	155	19	0	0	0	620	626	0	0	1089	344
Turn Type	Split	NA	Perm				Prot	NA			NA	Perm
Protected Phases	4	4					5	2			6	
Permitted Phases			4									6
Actuated Green, G (s)	14.2	14.2	14.2				19.0	57.8			34.8	34.8
Effective Green, g (s)	14.2	14.2	14.2				19.0	57.8			34.8	34.8
Actuated g/C Ratio	0.18	0.18	0.18				0.24	0.72			0.43	0.43
Clearance Time (s)	4.0	4.0	4.0				4.0	4.0			4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0				3.0	3.0			3.0	3.0
Lane Grp Cap (vph)	298	284	266				815	2556			1539	688
w/s Ratio Prot	c0.13	0.10					c0.18	0.18			c0.31	
w/s Ratio Perm			0.01									0.22
w/c Ratio	0.72	0.55	0.07				0.76	0.24			0.71	0.50
Uniform Delay, d1	31.1	30.0	27.4				28.4	3.7			18.4	16.3
Progression Factor	1.00	1.00	1.00				1.00	1.00			0.55	0.92
Incremental Delay, d2	8.5	2.1	0.1				4.2	0.2			1.1	1.1
Delay (s)	39.5	32.1	27.5				32.6	4.0			11.4	16.1
Level of Service	D	C	C				C	A			B	B
Approach Delay (s)		34.2				0.0		18.2			13.1	
Approach LOS		C				A		B			B	
Intersection Summary												
HCM 2000 Control Delay			18.1							B		
HCM 2000 Volume to Capacity ratio			0.73									
Actuated Cycle Length (s)			80.0							12.0		
Intersection Capacity Utilization			65.7%							C		
Analysis Period (min)			15									
c Critical Lane Group												

Dixon Ranch
20: Silva Valley Pkwy & WB US-50 Ramps

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↘	↘	↘	↘	↘	↘	↘	↘	↘
Volume (vph)	0	0	0	524	0	531	347	612	0	0	1063	516
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)				4.0	4.0	4.0	4.0	4.0			4.0	4.0
Lane Util. Factor				0.95	0.91	0.95	1.00	0.95			0.95	1.00
Frt				1.00	0.92	0.85	1.00	1.00			1.00	0.85
Flt Protected				0.95	0.98	1.00	0.95	1.00			1.00	1.00
Satd. Flow (prot)				1681	1520	1504	1770	3539			3539	1583
Flt Permitted				0.95	0.98	1.00	0.95	1.00			1.00	1.00
Satd. Flow (perm)				1681	1520	1504	1770	3539			3539	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	570	0	577	377	665	0	0	1155	561
RTOR Reduction (vph)	0	0	0	0	56	208	0	0	0	0	0	361
Lane Group Flow (vph)	0	0	0	399	328	156	377	665	0	0	1155	200
Turn Type				Split	NA	Perm	Prot	NA			NA	Perm
Protected Phases				8	8		5	2			6	
Permitted Phases						8						6
Actuated Green, G (s)				20.9	20.9	20.9	18.6	51.1			28.5	28.5
Effective Green, g (s)				20.9	20.9	20.9	18.6	51.1			28.5	28.5
Actuated g/C Ratio				0.26	0.26	0.26	0.23	0.64			0.36	0.36
Clearance Time (s)				4.0	4.0	4.0	4.0	4.0			4.0	4.0
Vehicle Extension (s)				3.0	3.0	3.0	3.0	3.0			3.0	3.0
Lane Grp Cap (vph)				439	397	392	411	2260			1260	563
v/s Ratio Prot				c0.24	0.22		c0.21	0.19			c0.33	
v/s Ratio Perm						0.10						0.13
w/c Ratio				0.91	0.83	0.40	0.92	0.29			0.92	0.35
Uniform Delay, d1				28.6	27.8	24.4	29.9	6.4			24.6	19.0
Progression Factor				1.00	1.00	1.00	1.20	0.97			0.83	0.37
Incremental Delay, d2				22.3	13.1	0.7	23.8	0.3			10.2	1.4
Delay (s)				50.9	40.9	25.0	59.7	6.6			30.6	8.4
Level of Service				D	D	C	E	A			C	A
Approach Delay (s)		0.0			39.4			25.8			23.4	
Approach LOS		A			D			C			C	
Intersection Summary												
HCM 2000 Control Delay				28.7								C
HCM 2000 Volume to Capacity ratio				0.91								
Actuated Cycle Length (s)				80.0			Sum of lost time (s)	12.0				
Intersection Capacity Utilization				81.1%			ICU Level of Service	D				
Analysis Period (min)				15								
c Critical Lane Group												


Dixon Ranch
21: Silva Valley Pkwy & County Club Dr.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↘	↘	↘	↘	↘
Volume (vph)	163	157	981	161	1	1416
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0			4.0
Lane Util. Factor	1.00	1.00	0.95			0.95
Frt	1.00	0.85	0.98			1.00
Flt Protected	0.95	1.00	1.00			0.95
Satd. Flow (prot)	1770	1583	3464			1770
Flt Permitted	0.95	1.00	1.00			0.95
Satd. Flow (perm)	1770	1583	3464			1770
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	177	171	1066	175	1	1539
RTOR Reduction (vph)	0	143	13	0	0	0
Lane Group Flow (vph)	177	28	1228	0	1	1539
Turn Type	NA	Perm	NA		Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8				
Actuated Green, G (s)	13.2	13.2	54.0		0.8	58.8
Effective Green, g (s)	13.2	13.2	54.0		0.8	58.8
Actuated g/C Ratio	0.16	0.16	0.68		0.01	0.73
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	292	261	2338		17	2601
v/s Ratio Prot	c0.10		0.35		0.00	c0.43
v/s Ratio Perm		0.02				
w/c Ratio	0.61	0.11	0.53		0.06	0.59
Uniform Delay, d1	31.0	28.4	6.5		39.2	5.0
Progression Factor	1.00	1.00	1.14		1.00	1.00
Incremental Delay, d2	3.5	0.2	0.8		1.5	1.0
Delay (s)	34.5	28.6	8.2		40.7	6.0
Level of Service	C	C	A		D	A
Approach Delay (s)	31.6		8.2			6.0
Approach LOS	C		A			A
Intersection Summary						
HCM 2000 Control Delay			9.7			HCM 2000 Level of Service A
HCM 2000 Volume to Capacity ratio			0.63			
Actuated Cycle Length (s)			80.0			Sum of lost time (s) 12.0
Intersection Capacity Utilization			54.8%			ICU Level of Service A
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch
24: Silva Valley Pkwy. & Appian Way


75-25 Modified Model, EPAP
Timing Plan: AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	37	1	87	155	2	63	22	212	46	26	254	21
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	40	1	95	168	2	68	24	230	50	28	276	23
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	136	239	304	327								
Volume Left (vph)	40	168	24	28								
Volume Right (vph)	95	68	50	23								
Hadj (s)	-0.32	0.00	-0.05	0.01								
Departure Headway (s)	5.9	5.9	5.6	5.6								
Degree Utilization, x	0.22	0.40	0.47	0.51								
Capacity (veh/h)	516	546	599	605								
Control Delay (s)	10.5	12.8	13.4	14.2								
Approach Delay (s)	10.5	12.8	13.4	14.2								
Approach LOS	B	B	B	B								
Intersection Summary												
Delay	13.1											
Level of Service	B											
Intersection Capacity Utilization	47.4%		ICU Level of Service	A								
Analysis Period (min)	15											

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↕			↕		↕
Volume (veh/h)	313	0	0	653	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	340	0	0	710	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)	766					
pX, platoon unblocked						
vC, conflicting volume				340	1050	340
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				340	1050	340
tC, single (s)				4.1	6.4	6.2
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				1219	252	702
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	340	710	0			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1700	1700			
Volume to Capacity	0.20	0.42	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS				A		
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS				A		
Intersection Summary						
Average Delay				0.0		
Intersection Capacity Utilization	37.7%		ICU Level of Service	A		
Analysis Period (min)	15					

Dixon Ranch
26: Site Dwy. Full/Site Dwy. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔	↔	↔	↔	↔
Volume (vph)	0	313	0	0	653	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0							
Lane Util. Factor		1.00			1.00							
Frt		1.00			1.00							
Flt Protected		1.00			1.00							
Satd. Flow (prot)		1863			1863							
Flt Permitted		1.00			1.00							
Satd. Flow (perm)		1863			1863							
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	340	0	0	710	0	0	0	0	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	340	0	0	710	0	0	0	0	0	0	0
Turn Type	Perm	NA		Perm	NA		Perm		Perm	Perm		
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4		4	8		
Actuated Green, G (s)		15.7			15.7							
Effective Green, g (s)		15.7			15.7							
Actuated g/C Ratio		1.00			1.00							
Clearance Time (s)		4.0			4.0							
Vehicle Extension (s)		3.0			3.0							
Lane Grp Cap (vph)		1863			1863							
v/s Ratio Prot		0.18			c0.38							
v/s Ratio Perm												
w/c Ratio		0.18			0.38							
Uniform Delay, d1		0.0			0.0							
Progression Factor		1.00			1.00							
Incremental Delay, d2		0.0			0.1							
Delay (s)		0.0			0.1							
Level of Service		A			A							
Approach Delay (s)		0.0			0.1			0.0			0.0	
Approach LOS		A			A			A			A	
Intersection Summary												
HCM 2000 Control Delay			0.1									A
HCM 2000 Volume to Capacity ratio			0.78									
Actuated Cycle Length (s)			15.7									8.0
Intersection Capacity Utilization			37.7%									A
Analysis Period (min)			15									
c Critical Lane Group												


Dixon Ranch
1: Green Valley Rd. & Francisco Rd.

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	426	893	314	150	535	68	348	252	17	105	210	200
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.7	5.7	4.0	5.7	4.0	5.7	4.0	4.5	4.0	4.5	4.5
Lane Util. Factor	0.97	0.95	1.00	1.00	0.95	1.00	0.97	0.95	0.99	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	1.00	0.85	1.00	0.99	1.00	1.00	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (prot)	3433	3539	1583	1770	3539	1583	3433	3506	1770	1863	1583	1583
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00	0.95	1.00	0.95	1.00	1.00	0.95
Satd. Flow (perm)	3433	3539	1583	1770	3539	1583	3433	3506	1770	1863	1583	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	463	971	341	163	582	74	378	274	18	114	228	217
RTOR Reduction (vph)	0	0	225	0	0	52	0	5	0	0	0	177
Lane Group Flow (vph)	463	971	116	163	582	22	378	287	0	114	228	40
Turn Type	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm	Prot	NA	Perm
Protected Phases	5	2		1	6		3	8		7	4	
Permitted Phases			2			6						4
Actuated Green, G (s)	13.1	28.0	28.0	9.1	24.0	24.0	11.7	19.8		7.1	15.2	15.2
Effective Green, g (s)	13.1	28.0	28.0	9.1	24.0	24.0	11.7	19.8		7.1	15.2	15.2
Actuated g/C Ratio	0.16	0.34	0.34	0.11	0.29	0.29	0.14	0.24		0.09	0.18	0.18
Clearance Time (s)	4.0	5.7	5.7	4.0	5.7	5.7	4.0	4.5		4.0	4.5	4.5
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	547	1205	539	195	1033	462	488	844		152	344	292
v/s Ratio Prot	c0.13	c0.27		0.09	0.16		c0.11	0.08		0.06	c0.12	
v/s Ratio Perm			0.07			0.01						0.03
w/c Ratio	0.85	0.81	0.22	0.84	0.56	0.05	0.77	0.34		0.75	0.66	0.14
Uniform Delay, d1	33.6	24.6	19.3	35.8	24.7	20.9	34.0	25.8		36.7	31.1	28.0
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	11.6	4.0	0.2	25.5	0.7	0.0	7.5	0.2		18.6	4.7	0.2
Delay (s)	45.1	28.7	19.5	61.3	25.4	20.9	41.5	26.0		55.3	35.9	28.2
Level of Service	D	C	B	E	C	C	D	C		E	D	C
Approach Delay (s)		31.2			32.1			34.8			36.9	
Approach LOS		C			C			C			D	
Intersection Summary												
HCM 2000 Control Delay					32.8							C
HCM 2000 Volume to Capacity ratio					0.79							
Actuated Cycle Length (s)					82.2							18.2
Intersection Capacity Utilization					69.1%							C
Analysis Period (min)					15							
c Critical Lane Group												

Dixon Ranch
2: El Dorado Hills Blvd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: PM




Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔		↔	↔		↔	↔	↔
Volume (vph)	138	957	24	51	567	92	55	214	92	59	101	109
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	3.5	6.0		3.5	6.0		4.0	4.0			5.5	5.5
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	1.00
Frt	1.00	1.00		1.00	0.98		1.00	0.95			1.00	0.85
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00			0.98	1.00
Satd. Flow (prot)	1770	1856		1770	1824		1770	1779			1829	1583
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00			0.98	1.00
Satd. Flow (perm)	1770	1856		1770	1824		1770	1779			1829	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	150	1040	26	55	616	100	60	233	100	64	110	118
RTOR Reduction (vph)	0	0	0	0	4	0	0	11	0	0	0	102
Lane Group Flow (vph)	150	1066	0	55	712	0	60	322	0	0	174	16
Turn Type	Prot	NA		Prot	NA		Split	NA		Split	NA	Perm
Protected Phases	1	6		5	2		4	4		3	3	
Permitted Phases												3
Actuated Green, G (s)	13.7	77.5		4.5	68.3		22.0	22.0			18.9	18.9
Effective Green, g (s)	13.7	77.5		4.5	68.3		22.0	22.0			18.9	18.9
Actuated g/C Ratio	0.10	0.55		0.03	0.48		0.16	0.16			0.13	0.13
Clearance Time (s)	3.5	6.0		3.5	6.0		4.0	4.0			5.5	5.5
Vehicle Extension (s)	2.5	5.0		2.5	5.0		4.5	4.5			4.5	4.5
Lane Grp Cap (vph)	170	1013		56	877		274	275			243	210
w/s Ratio Prot	c0.08	c0.57		0.03	0.39		0.03	c0.18			c0.10	
w/s Ratio Perm												0.01
w/c Ratio	0.88	1.05		0.98	0.81		0.22	1.17			0.72	0.07
Uniform Delay, d1	63.3	32.2		68.7	31.3		52.4	60.0			58.9	53.8
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00			1.00	1.00
Incremental Delay, d2	37.4	42.9		113.3	6.5		0.7	108.7			10.9	0.3
Delay (s)	100.7	75.1		182.0	37.8		53.1	168.6			69.8	54.1
Level of Service	F	E		F	D		D	F			E	D
Approach Delay (s)		78.3			48.1			151.0			63.5	
Approach LOS		E			D			F			E	

Intersection Summary			
HCM 2000 Control Delay	78.7	HCM 2000 Level of Service	E
HCM 2000 Volume to Capacity ratio	1.02		
Actuated Cycle Length (s)	141.9	Sum of lost time (s)	19.0
Intersection Capacity Utilization	96.9%	ICU Level of Service	F
Analysis Period (min)	15		

c Critical Lane Group

Dixon Ranch
3: Silva Valley Pkwy. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: PM




Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔		↔	↔		↔	↔	↔
Volume (vph)	9	713	382	64	420	3	282	18	106	2	9	4
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6			4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	0.87			0.97	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00			0.99	
Satd. Flow (prot)	1770	1863	1583	1770	1861		1770	1625			1789	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00			0.99	
Satd. Flow (perm)	1770	1863	1583	1770	1861		1770	1625			1789	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	10	775	415	70	457	3	307	20	115	2	10	4
RTOR Reduction (vph)	0	0	132	0	0	0	0	91	0	0	4	0
Lane Group Flow (vph)	10	775	283	70	460	0	307	44	0	0	12	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA		Split	NA	
Protected Phases	1	6		5	2		8	8		4	4	
Permitted Phases			6									
Actuated Green, G (s)	0.7	48.7	48.7	5.0	53.0		19.5	19.5			2.5	
Effective Green, g (s)	0.7	48.7	48.7	5.0	53.0		19.5	19.5			2.5	
Actuated g/C Ratio	0.01	0.52	0.52	0.05	0.56		0.21	0.21			0.03	
Clearance Time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6			4.0	
Vehicle Extension (s)	2.5	3.0	3.0	2.5	3.0		2.5	2.5			2.5	
Lane Grp Cap (vph)	13	965	820	94	1049		367	337			47	
w/s Ratio Prot	0.01	c0.42		c0.04	0.25		c0.17	0.03			c0.01	
w/s Ratio Perm			0.18									
w/c Ratio	0.77	0.80	0.35	0.74	0.44		0.84	0.13			0.26	
Uniform Delay, d1	46.6	18.7	13.3	43.9	11.9		35.7	30.3			44.8	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2	124.9	4.9	0.3	25.7	0.3		14.9	0.1			2.1	
Delay (s)	171.5	23.6	13.5	69.6	12.2		50.6	30.5			47.0	
Level of Service	F	C	B	E	B		D	C			D	
Approach Delay (s)		21.4			19.8			44.5			47.0	
Approach LOS		C			B			D			D	

Intersection Summary			
HCM 2000 Control Delay	25.8	HCM 2000 Level of Service	C
HCM 2000 Volume to Capacity ratio	0.79		
Actuated Cycle Length (s)	94.0	Sum of lost time (s)	18.3
Intersection Capacity Utilization	75.3%	ICU Level of Service	D
Analysis Period (min)	15		

c Critical Lane Group

Dixon Ranch
4: Loch Way & Green Valley Rd

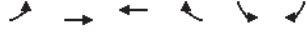
75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (veh/h)	810	27	4	453	21	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	880	29	4	492	23	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			910		1396	895
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			910		1396	895
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		85	99
cM capacity (veh/h)			749		155	339
Direction, Lane #	EB 1	WB 1	NB 1	NB 2		
Volume Total	910	497	23	4		
Volume Left	0	4	23	0		
Volume Right	29	0	0	4		
cSH	1700	749	155	339		
Volume to Capacity	0.54	0.01	0.15	0.01		
Queue Length 95th (ft)	0	0	13	1		
Control Delay (s)	0.0	0.2	32.3	15.7		
Lane LOS		A	D	C		
Approach Delay (s)	0.0	0.2	29.6			
Approach LOS			D			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			54.3%		ICU Level of Service	A
Analysis Period (min)			15			

Dixon Ranch
5: Green Valley Rd & Wilson Estates

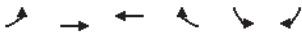
75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	31	783	438	12	8	19
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	851	476	13	9	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	489				1401	483
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	489				1401	483
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				94	96
cM capacity (veh/h)	1074				150	584
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	885	489	29			
Volume Left	34	0	9			
Volume Right	0	13	21			
cSH	1074	1700	314			
Volume to Capacity	0.03	0.29	0.09			
Queue Length 95th (ft)	2	0	8			
Control Delay (s)	0.8	0.0	17.6			
Lane LOS		A	C			
Approach Delay (s)	0.8	0.0	17.6			
Approach LOS			C			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization			76.3%		ICU Level of Service	D
Analysis Period (min)			15			

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd


75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	12	807	449	5	10	14
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	877	488	5	11	15
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	493				1394	491
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	493				1394	491
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				93	97
cM capacity (veh/h)	1070				154	578
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	890	493	26			
Volume Left	13	0	11			
Volume Right	0	5	15			
cSH	1070	1700	269			
Volume to Capacity	0.01	0.29	0.10			
Queue Length 95th (ft)	1	0	8			
Control Delay (s)	0.3	0.0	19.8			
Lane LOS	A		C			
Approach Delay (s)	0.3	0.0	19.8			
Approach LOS			C			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization		62.1%		ICU Level of Service		B
Analysis Period (min)		15				

Dixon Ranch
7: Deer Valley Rd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔				↔	↔	↔	↔
Volume (veh/h)	45	730	18	39	416	7	8	1	23	7	0	14
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	793	20	42	452	8	9	1	25	8	0	15
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	460			813			1453	1446	803	1458	1452	456
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	460			813			1453	1446	803	1458	1452	456
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	96			95			91	99	93	92	100	97
cM capacity (veh/h)	1101			814			98	119	383	93	118	604
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	49	813	42	460	35	23						
Volume Left	49	0	42	0	9	8						
Volume Right	0	20	0	8	25	15						
cSH	1101	1700	814	1700	213	213						
Volume to Capacity	0.04	0.48	0.05	0.27	0.16	0.11						
Queue Length 95th (ft)	3	0	4	0	14	9						
Control Delay (s)	8.4	0.0	9.7	0.0	25.2	24.0						
Lane LOS	A		A		D	C						
Approach Delay (s)	0.5		0.8		25.2	24.0						
Approach LOS					D	C						
Intersection Summary												
Average Delay					1.6							
Intersection Capacity Utilization		49.5%			ICU Level of Service		A					
Analysis Period (min)		15										

Dixon Ranch
8: Silver Springs Pkwy & Green Valley Rd

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (vph)	700	114	29	373	67	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0			4.0	4.0	
Lane Util. Factor	1.00			1.00	1.00	
Frt	0.98			1.00	0.97	
Flt Protected	1.00			1.00	0.96	
Satd. Flow (prot)	1828			1856	1743	
Flt Permitted	1.00			0.92	0.96	
Satd. Flow (perm)	1828			1720	1743	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	761	124	32	405	73	18
RTOR Reduction (vph)	9	0	0	0	15	0
Lane Group Flow (vph)	876	0	0	437	76	0
Turn Type	NA		Perm	NA	NA	
Protected Phases	4			8	2	
Permitted Phases			8			
Actuated Green, G (s)	25.9			25.9	7.7	
Effective Green, g (s)	25.9			25.9	7.7	
Actuated g/C Ratio	0.62			0.62	0.19	
Clearance Time (s)	4.0			4.0	4.0	
Vehicle Extension (s)	3.0			3.0	3.0	
Lane Grp Cap (vph)	1138			1070	322	
w/s Ratio Prot	c0.48				c0.04	
w/s Ratio Perm				0.25		
w/c Ratio	0.77			0.41	0.24	
Uniform Delay, d1	5.7			4.0	14.4	
Progression Factor	1.00			1.00	1.00	
Incremental Delay, d2	3.2			0.3	0.4	
Delay (s)	8.9			4.2	14.8	
Level of Service	A			A	B	
Approach Delay (s)	8.9			4.2	14.8	
Approach LOS	A			A	B	
Intersection Summary						
HCM 2000 Control Delay			7.8			HCM 2000 Level of Service A
HCM 2000 Volume to Capacity ratio			0.65			
Actuated Cycle Length (s)			41.6			Sum of lost time (s) 8.0
Intersection Capacity Utilization			55.2%			ICU Level of Service B
Analysis Period (min)			15			
c Critical Lane Group						


Dixon Ranch
9: Bass Lake Rd. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	2	563	117	111	302	8	65	6	192	17	7	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.97		1.00	1.00		1.00	0.85	0.95			
Flt Protected	0.95	1.00		0.95	1.00		0.96	1.00	0.98			
Satd. Flow (prot)	1770	1815		1770	1855		1782	1583	1739			
Flt Permitted	0.95	1.00		0.95	1.00		0.96	1.00	0.98			
Satd. Flow (perm)	1770	1815		1770	1855		1782	1583	1739			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	2	612	127	121	328	9	71	7	209	18	8	13
RTOR Reduction (vph)	0	7	0	0	1	0	0	0	185	0	12	0
Lane Group Flow (vph)	2	732	0	121	336	0	0	78	24	0	27	0
Turn Type	Prot	NA		Prot	NA		Split	NA	Perm	Split	NA	
Protected Phases	7	4		3	8		2	2		6	6	
Permitted Phases									2			
Actuated Green, G (s)	0.7	38.8		7.1	45.2		8.4	8.4	4.2			
Effective Green, g (s)	0.7	38.8		7.1	45.2		8.4	8.4	4.2			
Actuated g/C Ratio	0.01	0.52		0.10	0.61		0.11	0.11	0.06			
Clearance Time (s)	4.0	4.0		4.0	4.0		4.0	4.0	4.0			
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0			
Lane Grp Cap (vph)	16	945		168	1125		200	178	98			
w/s Ratio Prot	0.00	c0.40		c0.07	0.18		c0.04		c0.02			
w/s Ratio Perm								0.01				
w/c Ratio	0.12	0.77		0.72	0.30		0.39	0.13	0.27			
Uniform Delay, d1	36.6	14.3		32.7	7.0		30.7	29.8	33.7			
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00			
Incremental Delay, d2	3.5	4.0		14.1	0.1		1.3	0.3	1.5			
Delay (s)	40.1	18.4		46.8	7.2		31.9	30.1	35.2			
Level of Service	D	B		D	A		C	C	D			
Approach Delay (s)		18.4			17.7		30.6		35.2			
Approach LOS		B			B		C		D			
Intersection Summary												
HCM 2000 Control Delay					20.9							HCM 2000 Level of Service C
HCM 2000 Volume to Capacity ratio					0.68							
Actuated Cycle Length (s)					74.5				16.0			Sum of lost time (s)
Intersection Capacity Utilization					62.0%							ICU Level of Service B
Analysis Period (min)					15							
c Critical Lane Group												

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr.


75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↗		↕	↘	↕	↖		↕	↗	↘
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	0	41	497	26	36	40	535	371	19	9	202	2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	45	540	28	39	43	582	403	21	10	220	2
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	45	540	111	582	424	10	222					
Volume Left (vph)	0	0	28	582	0	10	0					
Volume Right (vph)	0	540	43	0	21	0	2					
Hadj (s)	0.03	-0.57	-0.15	0.53	0.00	0.53	0.03					
Departure Headway (s)	6.6	3.2	6.2	5.8	5.3	6.8	6.3					
Degree Utilization, x	0.08	0.48	0.19	0.94	0.63	0.02	0.39					
Capacity (veh/h)	519	1116	561	607	671	510	557					
Control Delay (s)	10.1	9.1	10.6	46.6	15.5	8.7	12.0					
Approach Delay (s)	9.2		10.6	33.5	11.9							
Approach LOS	A		B	D	B							
Intersection Summary												
Delay			22.2									
Level of Service			C									
Intersection Capacity Utilization			62.8%		ICU Level of Service		B					
Analysis Period (min)			15									

Dixon Ranch
13: El Dorado Hills Blvd. & Harvard Way

75-25 Modified Model, EPAP
Timing Plan: PM



Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	↕	↗	↕	↘	↕	↗	
Volume (vph)	141	125	939	184	162	585	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0	
Lane Util. Factor	1.00	1.00	0.95		0.97	0.95	
Frt	1.00	0.85	0.98		1.00	1.00	
Flt Protected	0.95	1.00	1.00		0.95	1.00	
Satd. Flow (prot)	1770	1583	3452		3433	3539	
Flt Permitted	0.95	1.00	1.00		0.95	1.00	
Satd. Flow (perm)	1770	1583	3452		3433	3539	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	153	136	1021	200	176	636	
RTOR Reduction (vph)	0	115	23	0	0	0	
Lane Group Flow (vph)	153	21	1198	0	176	636	
Turn Type	NA	Perm	NA		Prot	NA	
Protected Phases	8		2		1	6	
Permitted Phases	8						
Actuated Green, G (s)	7.9	7.9	27.1		3.7	34.8	
Effective Green, g (s)	7.9	7.9	27.1		3.7	34.8	
Actuated g/C Ratio	0.16	0.16	0.53		0.07	0.69	
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	275	246	1845		250	2429	
v/s Ratio Prot	c0.09		c0.35		c0.05	0.18	
v/s Ratio Perm	0.01						
w/c Ratio	0.56	0.09	0.65		0.70	0.26	
Uniform Delay, d1	19.8	18.3	8.4		23.0	3.0	
Progression Factor	1.00	1.00	1.00		1.00	1.00	
Incremental Delay, d2	2.4	0.2	0.8		8.7	0.1	
Delay (s)	22.2	18.5	9.2		31.6	3.1	
Level of Service	C	B	A		C	A	
Approach Delay (s)	20.4		9.2			9.3	
Approach LOS	C		A			A	
Intersection Summary							
HCM 2000 Control Delay			10.6		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio			0.64				
Actuated Cycle Length (s)			50.7		Sum of lost time (s)		12.0
Intersection Capacity Utilization			54.3%		ICU Level of Service		A
Analysis Period (min)			15				
c Critical Lane Group							

Dixon Ranch
21: Silva Valley Pkwy & County Club Dr.

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↘	↗	↕		↘	↗
Volume (vph)	32	205	1348	263	0	1288
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0			4.0
Lane Util. Factor	1.00	1.00	0.95			0.95
Frt	1.00	0.85	0.98			1.00
Flt Protected	0.95	1.00	1.00			1.00
Satd. Flow (prot)	1770	1583	3453			3539
Flt Permitted	0.95	1.00	1.00			1.00
Satd. Flow (perm)	1770	1583	3453			3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	35	223	1465	286	0	1400
RTOR Reduction (vph)	0	98	9	0	0	0
Lane Group Flow (vph)	35	125	1742	0	0	1400
Turn Type	NA	Perm	NA		Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8				
Actuated Green, G (s)	12.9	12.9	79.1			79.1
Effective Green, g (s)	12.9	12.9	79.1			79.1
Actuated g/C Ratio	0.13	0.13	0.79			0.79
Clearance Time (s)	4.0	4.0	4.0			4.0
Vehicle Extension (s)	3.0	3.0	3.0			3.0
Lane Grp Cap (vph)	228	204	2731			2799
v/s Ratio Prot	0.02		c0.50			0.40
v/s Ratio Perm		c0.08				
w/c Ratio	0.15	0.61	0.64			0.50
Uniform Delay, d1	38.7	41.2	4.4			3.6
Progression Factor	1.00	1.00	0.91			1.00
Incremental Delay, d2	0.3	5.4	0.8			0.6
Delay (s)	39.0	46.6	4.9			4.3
Level of Service	D	D	A			A
Approach Delay (s)	45.6		4.9			4.3
Approach LOS	D		A			A
Intersection Summary						
HCM 2000 Control Delay			7.7			HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio			0.66			A
Actuated Cycle Length (s)			100.0			Sum of lost time (s)
Intersection Capacity Utilization			65.0%			12.0
Analysis Period (min)			15			ICU Level of Service
						C
c Critical Lane Group						

Dixon Ranch
22: Silva Valley Pkwy. & Serrano Pkwy.

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↘	↗	↕	↘	↗	↕	↘	↗	↕	↘	↗	↕
Volume (vph)	175	227	175	361	111	145	192	692	374	223	443	112
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.3		4.0	5.3		4.0	5.3	5.3	4.0	5.3	
Lane Util. Factor	1.00	0.95		1.00	0.95		1.00	0.95	1.00	1.00	0.95	
Frt	1.00	0.93		1.00	0.92		1.00	1.00	0.85	1.00	0.97	
Flt Protected	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1770	3308		1770	3239		1770	3539	1583	1770	3432	
Flt Permitted	0.95	1.00		0.95	1.00		0.95	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1770	3308		1770	3239		1770	3539	1583	1770	3432	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	190	247	190	392	121	158	209	752	407	242	482	122
RTOR Reduction (vph)	0	157	0	0	124	0	0	0	294	0	21	0
Lane Group Flow (vph)	190	280	0	392	155	0	209	752	113	242	583	0
Turn Type	Prot	NA		Prot	NA		Prot	NA	Perm	Prot	NA	
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases									2			
Actuated Green, G (s)	14.1	13.1		20.0	19.0		11.0	23.5	23.5	13.0	25.5	
Effective Green, g (s)	14.1	13.1		20.0	19.0		11.0	23.5	23.5	13.0	25.5	
Actuated g/C Ratio	0.16	0.15		0.23	0.22		0.12	0.27	0.27	0.15	0.29	
Clearance Time (s)	4.0	5.3		4.0	5.3		4.0	5.3	5.3	4.0	5.3	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	282	491		401	697		220	942	421	260	992	
v/s Ratio Prot	0.11	c0.08		c0.22	0.05		0.12	c0.21		c0.14	0.17	
v/s Ratio Perm									0.07			
w/c Ratio	0.67	0.57		0.98	0.22		0.95	0.80	0.27	0.93	0.59	
Uniform Delay, d1	34.9	34.9		33.9	28.5		38.3	30.1	25.6	37.2	26.8	
Progression Factor	1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	6.2	1.6		38.6	0.2		46.1	4.8	0.3	37.5	0.9	
Delay (s)	41.1	36.5		72.5	28.7		84.4	34.9	25.9	74.7	27.7	
Level of Service	D	D		E	C		F	C	C	E	C	
Approach Delay (s)		37.9			54.3			39.8			41.2	
Approach LOS		D			D			D			D	
Intersection Summary												
HCM 2000 Control Delay				42.6								HCM 2000 Level of Service
HCM 2000 Volume to Capacity ratio				0.82								D
Actuated Cycle Length (s)				88.2								Sum of lost time (s)
Intersection Capacity Utilization				78.9%								18.6
Analysis Period (min)				15								ICU Level of Service
												D
c Critical Lane Group												

Dixon Ranch
23: Harvard Way & Silva Valley Pkwy.

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↔	↔	↕	↔	↔	↕	↔	↔	↕	↔
Volume (vph)	133	11	203	9	11	6	198	318	11	10	223	77
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.94	1.00	0.99	1.00	1.00	1.00	0.85	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1863	1583	1770	1760	1770	1853	1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	1863	1583	1770	1760	1770	1853	1770	1863	1583	1770	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	145	12	221	10	12	7	215	346	12	11	242	84
RTOR Reduction (vph)	0	0	194	0	7	0	0	1	0	0	0	58
Lane Group Flow (vph)	145	12	27	10	12	0	215	357	0	11	242	26
Turn Type	Prot	NA	Perm	Prot	NA	Prot	NA	Prot	NA	Prot	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases			4									6
Actuated Green, G (s)	5.2	5.8	5.8	0.6	1.2		10.1	24.2		0.6	14.7	14.7
Effective Green, g (s)	5.2	5.8	5.8	0.6	1.2		10.1	24.2		0.6	14.7	14.7
Actuated g/C Ratio	0.11	0.12	0.12	0.01	0.03		0.21	0.51		0.01	0.31	0.31
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	3.0
Lane Grp Cap (vph)	195	228	194	22	44		378	950		22	580	493
v/s Ratio Prot	c0.08	0.01		0.01	0.01		c0.12	c0.19		0.01	0.13	
v/s Ratio Perm			c0.02									0.02
v/c Ratio	0.74	0.05	0.14	0.45	0.28		0.57	0.38		0.50	0.42	0.05
Uniform Delay, d1	20.4	18.3	18.5	23.1	22.6		16.6	6.9		23.2	12.9	11.4
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	1.00
Incremental Delay, d2	14.2	0.1	0.3	14.2	3.4		2.0	0.3		16.8	0.5	0.0
Delay (s)	34.6	18.4	18.8	37.3	26.0		18.6	7.2		39.9	13.3	11.4
Level of Service	C	B	B	D	C		B	A		D	B	B
Approach Delay (s)		24.8			29.9			11.5				13.7
Approach LOS		C			C			B				B
Intersection Summary												
HCM 2000 Control Delay			16.3				HCM 2000 Level of Service					B
HCM 2000 Volume to Capacity ratio			0.53									
Actuated Cycle Length (s)			47.2				Sum of lost time (s)					16.0
Intersection Capacity Utilization			46.7%				ICU Level of Service					A
Analysis Period (min)			15									
c Critical Lane Group												


Dixon Ranch
24: Silva Valley Pkwy. & Appian Way

75-25 Modified Model, EPAP
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↕	↔	↔	↕	↔	↔	↕	↔	↔	↕	↔
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	17	4	40	56	2	43	76	264	97	52	213	32
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	18	4	43	61	2	47	83	287	105	57	232	35
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	66	110	475	323								
Volume Left (vph)	18	61	83	57								
Volume Right (vph)	43	47	105	35								
Hadj (s)	-0.30	-0.11	-0.06	0.00								
Departure Headway (s)	5.8	5.9	4.8	5.1								
Degree Utilization, x	0.11	0.18	0.63	0.45								
Capacity (veh/h)	514	532	722	680								
Control Delay (s)	9.5	10.1	15.8	12.2								
Approach Delay (s)	9.5	10.1	15.8	12.2								
Approach LOS	A	B	C	B								
Intersection Summary												
Delay			13.5									
Level of Service			B									
Intersection Capacity Utilization			49.7%		ICU Level of Service							A
Analysis Period (min)			15									

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.


75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔			↑		↔
Volume (veh/h)	778	0	0	435	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	846	0	0	473	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)				716		
pX, platoon unblocked						
vC, conflicting volume			846		1318	846
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			846		1318	846
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			100		100	100
cM capacity (veh/h)			791		173	362
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	846	473	0			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1700	1700			
Volume to Capacity	0.50	0.28	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS			A			
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS			A			
Intersection Summary						
Average Delay		0.0				
Intersection Capacity Utilization		44.3%	ICU Level of Service	A		
Analysis Period (min)		15				

Dixon Ranch
26: Site Dwy, Full/Site Dwy. & Green Valley Rd.

75-25 Modified Model, EPAP
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔	↔	↔	↔	↔
Volume (vph)	0	778	0	0	435	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0			4.0							
Lane Util. Factor		1.00			1.00							
Frt		1.00			1.00							
Flt Protected		1.00			1.00							
Satd. Flow (prot)		1863			1863							
Flt Permitted		1.00			1.00							
Satd. Flow (perm)		1863			1863							
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	846	0	0	473	0	0	0	0	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	846	0	0	473	0	0	0	0	0	0	0
Turn Type	Perm	NA		Perm	NA		Perm		Perm	Perm		
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4		4	8		
Actuated Green, G (s)		17.9			17.9							
Effective Green, g (s)		17.9			17.9							
Actuated g/C Ratio		1.00			1.00							
Clearance Time (s)		4.0			4.0							
Vehicle Extension (s)		3.0			3.0							
Lane Grp Cap (vph)		1863			1863							
v/s Ratio Prot		c0.45			0.25							
v/s Ratio Perm												
w/c Ratio		0.45			0.25							
Uniform Delay, d1		0.0			0.0							
Progression Factor		1.00			1.00							
Incremental Delay, d2		0.2			0.1							
Delay (s)		0.2			0.1							
Level of Service		A			A							
Approach Delay (s)		0.2			0.1			0.0				0.0
Approach LOS		A			A			A				A
Intersection Summary												
HCM 2000 Control Delay					0.1			HCM 2000 Level of Service	A			
HCM 2000 Volume to Capacity ratio					0.82							
Actuated Cycle Length (s)					17.9			Sum of lost time (s)	8.0			
Intersection Capacity Utilization					44.3%			ICU Level of Service	A			
Analysis Period (min)					15							
c Critical Lane Group												

Basic

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				Existing plus Approved Projects Conditions														
				Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures							
	Length (ft)	Number of Lanes (N)	Interchange Density (I/mi)	AM Peak	PM Peak	V _b	FFS	S	D	LOS	V _b	FFS	S	D	LOS			
				(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)					
Eastbound	West of El Dorado Hills Blvd	8180	4	0.67	3,081	5,593	862.345	73.11	75	74.7902	11.53	B	1565.432	73.11	75	71.4608	21.906	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	1490	4	0.67	1,146	3,139	320.755	73.11	75	69.8926	4.5893	A	878.5788	73.11	75	74.8368	11.74	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy SB On Ramp	1130	4	0.50	1,517	3,619	424.595	73.6	75	71.3348	5.9521	A	1012.927	73.6	75	74.9982	13.506	B
	East of Silva Valley Pkwy	1980	4	0.50	2,672	5,210	747.87	73.6	75	74.2963	10.066	A	1458.234	73.6	75	72.6755	20.065	C
Westbound	East of Silva Valley Pkwy	1950	3	0.50	5,133	2,947	1915.58	73.6	75	65.7202	29.147	D	1099.786	73.6	75	74.8898	14.685	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy NB On Ramp	1640	3	0.50	4,078	1,507	1521.86	73.6	75	71.9852	21.141	C	562.3949	73.6	75	72.8801	7.7167	A
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	3080	3	0.67	3,943	1,590	1471.48	73.11	75	72.5392	20.285	C	593.3696	73.11	75	73.1696	8.1095	A
	West of El Dorado Hills Blvd	6320	3	0.67	5,754	3,153	2147.33	73.11	75	60.4279	35.535	E	1176.663	73.11	75	74.6545	15.761	B

Universal Inputs:
 PHF 0.92
 (P_v) 6%
 f_{hv} 0.970873786

Merge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

										Existing plus Approve										
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures													
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/ln)						(pc/mi/ln)		
EB	Silva Valley Pkwy SB On Ramp	4	1	200	2672	1517	1155	2991	1698	1293	49	0.1199	203.6	9600	747	153	679	0.3116	19.011	B
	Silva Valley Pkwy NB On Ramp	4	1	1000	2672	2672	0	2991	2991	0	85	0.2178	651.54	9600	1170	489	1197	0.3116	8.5384	A
WB	El Dorado Hills Blvd On Ramp	3	1	800	5754	3943	1811	6442	4414.4	2027.5	126	0.5999	2648.2	7200	1766	1986	2648	0.8947	35.997	E

Universal Inputs:

Length	1500	(ft)
S _{FR}	70	(mi/h)
S _{RA}	35	(mi/h)
PHF	0.92	
(P _c)	6%	
f _{drv}	0.970873786	

Merge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs			Project Conditions																	
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	PM Flow Inputs			PM LOS Performance Measures													
				Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/hr)	(pc/h)				(pc/h/in)					(pc/mi/in)		
EB	Silva Valley Pkwy SB On Ramp	4	1	200	5210	3619	1591	5833	4052	1781	116	-0.0049	-19.665	9600	2036	-15	1621	0.6076	29.937	D
	Silva Valley Pkwy NB On Ramp	4	1	1000	5210	5210	0	5833	5833	0	167	0.2178	1270.4	9600	2281	953	2333	0.6076	17.404	B
WB	El Dorado Hills Blvd On Ramp	3	1	800	3153	1590	1563	3530	1780.1	1749.9	51	0.5999	1067.9	7200	712	801	1068	0.4903	21.633	C
Universal Inputs:																				
Length	1500		(ft)																	
S _{FF}	70		(mi/h)																	
S _{FR}	35		(mi/h)																	
PHF	0.92																			
(P _r)	6%																			
f _{HW}	0.970873786																			

Diverge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				AM Flow Inputs			AM LOS Performance Measures										Existing plus Approved I		
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume	Upstream Volume	Ramp Volume	V _D	V _F	V _R	P _{FD}	V ₁₂	Capacity	V ₃	V _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)					(pc/mi/ln)			
EB	Latrobe SB Off Ramp	4	1	140	1599	3081	1482	1790.18	3449.4	1659.2	0.436	2439.7	9600	505	1830	2440	0.3593	23.974	C
	Latrobe NB Off Ramp	4	1	120	1146	1599	453	1283.02	1790.2	507.16	0.436	1066.6	9600	362	800	1067	0.1865	12.344	B
WB	Silva Valley Pkwy Off Ramp	3	2	1300	4078	5133	1055	4565.59	5746.7	1181.1	0.562	3747	7200	2000	2810	3747	0.7982	24.776	C

Universal Inputs:
 Length 1500 (ft)
 S_{FF} 70 (mi/h)
 S_{FR} 35 (mi/h)
 PHF 0.92
 P_t 6%
 F_{RV} 0.970873786

Diverge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				Projects Conditions															
				PM Flow Inputs			PM LOS Performance Measures												
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	P _{FD}	V ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)						(pc/mi/ln)		
EB	Latrobe SB Off Ramp	4	1	140	4170	5593	1423	4668.59	6261.7	1593.1	0.436	3628.6	9600	1317	2721	3629	0.6523	34.198	D
	Latrobe NB Off Ramp	4	1	120	3139	4170	1031	3514.32	4668.6	1154.3	0.436	2686.5	9600	991	2015	2687	0.4863	26.276	C
WB	Silva Valley Pkwy Off Ramp	3	2	1300	1507	2947	1440	1687.18	3299.4	1612.2	0.6034	2630.1	7200	669	1973	2630	0.4582	15.171	B

Universal Inputs:
 Leng_D 1500 (ft)
 S_{FF} 70 (mi/h)
 S_{FR} 35 (mi/h)
 PHF 0.92
 P₁ 6%
 F_{RV} 0.970873786

Weave

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Basic Freeway LOS Default Values Segment	Existing Plus Approved Projects		Existing Plus Approved Projects plus Proposed	
	EB	WB	EB	WB
	E	D	E	D
Segment	Latrobe Rd On Ramp to Silva Valley Pkwy Off Ramp	Silva Valley Pkwy NB On Ramp to El Dorado Hills Blvd Off Ramp	Latrobe Rd On Ramp to Silva Valley Pkwy Off Ramp	Silva Valley Pkwy NB On Ramp to El Dorado Hills Blvd Off Ramp
Segment Inputs				
Length (ft)	3450	4150	3450	4150
Number of lanes (N) ^c	5	4	5	4
Exist Lanes	1	1	1	1
L _i (ft)	2450	3150	2450	3150
Interchange Density (I/mi)	0.67	0.67	0.67	0.67
LC _{RF} <i>minimum number of lane changes</i>	0	1	0	1
LC _{FR}	1	1	1	1
LC _{RR}	0	0	0	0
AM Flow Inputs				
V _U (veh/h) <i>total freeway volume upstream</i>	1146	4078	1161	4078
V _{ONR} (veh/h) <i>total on ramp volume</i>	863	863	863	905
V _{OFFR} (veh/h) <i>total on ramp volume</i>	492	998	507	998
V _{FF} <i>freeway to freeway</i>	865.3469388	3254.311678	870.1763834	3261.254264
V _{RF} <i>ramp to freeway</i>	651.6530612	688.6883222	646.8236166	723.7457355
V _{FR} <i>freeway to ramp</i>	280.6530612	823.6883222	290.8236166	816.7457355
V _{RR} <i>ramp to ramp</i>	211.3469388	174.3116778	216.1763834	181.2542645
PHF	0.92	0.92	0.92	0.92
% Trucks (P _T)	6%	6%	6%	6%
f _{HV}	0.970873786	0.970873786	0.970873786	0.970873786
V _{FF} <i>freeway to freeway (pc/h/l)</i>	968.8123336	3643.414161	974.2192118	3651.18684
V _{RF} <i>ramp to freeway (pc/h/l)</i>	729.5681012	771.0314912	724.1612229	810.2805517
V _{FR} <i>freeway to ramp (pc/h/l)</i>	314.2094055	922.1727955	325.5960055	914.4001169
V _{RR} <i>ramp to ramp (pc/h/l)</i>	236.6166815	195.1532914	242.0235597	202.92597
v _W <i>weaving demand flow</i>	1043.777507	1693.204287	1049.757228	1724.680669
v _{NW} <i>nonweaving demand flow</i>	1205.429015	3838.567452	1216.242772	3854.11281
v <i>total demand flow</i>	2249.206522	5531.771739	2266	5578.793478
VR	0.46406477	0.306087157	0.463264443	0.309149402
N _{WL}	2	2	2	2
L _{MAX}	7409.428651	5649.292911	7400.21026	5682.257789
	YES	YES	YES	YES
AM LOS Performance Measures				
c _W <i>capacity under prevailing conditions (veh/h)</i>	5015	7609	5029	7533
v/c	0.435432832	0.70582891	0.437462716	0.719010268
S _W (mi/h)	62.3	58.3	62.3	58.3
S _{NW} (mi/h)	65.6	51.2	65.5	50.9
S (mi/h)	64.02615516	53.18245474	63.977634	52.97890455
D (pc/mi/ln)	7.025899076	26.00374393	7.08372554	26.32554186
LOS	A	C	A	C
PM Flow Inputs				
V _U (veh/h) <i>total freeway volume upstream</i>	3139	1507	3185	1507
V _{ONR} (veh/h) <i>total on ramp volume</i>	1146	667	1146	694
V _{OFFR} (veh/h) <i>total on ramp volume</i>	666	584	712	584
V _{FF} <i>freeway to freeway</i>	2651.118086	1102.175713	2661.39806	1107.141754
V _{RF} <i>ramp to freeway</i>	967.8819137	487.824287	957.6019395	509.8582463
V _{FR} <i>freeway to ramp</i>	487.8819137	404.824287	523.6019395	399.8582463
V _{RR} <i>ramp to ramp</i>	178.1180863	179.175713	188.3980605	184.1417537
PHF	0.92	0.92	0.92	0.92
% Trucks (P _T)	6%	6%	6%	6%
f _{HV}	0.970873786	0.970873786	0.970873786	0.970873786
V _{FF} <i>freeway to freeway (pc/h/l)</i>	2968.099597	1233.957592	2979.608698	1239.517398
V _{RF} <i>ramp to freeway (pc/h/l)</i>	1083.606925	546.151104	1072.097824	570.8195583
V _{FR} <i>freeway to ramp (pc/h/l)</i>	546.2156207	453.2271909	586.2065192	447.6673844
V _{RR} <i>ramp to ramp (pc/h/l)</i>	199.4148141	200.598896	210.9239156	206.1587026
v _W <i>weaving demand flow</i>	1629.822546	999.3782949	1658.304343	1018.486943

v_{NW} nonweaving demand flow	3167.514411	1434.556488	3190.532614	1445.676101
v total demand flow	4797.336957	2433.934783	4848.836957	2464.163043
VR	0.339734849	0.41060192	0.342000434	0.413319624
N_{WL}	2	2	2	2
L_{MAX}	6014.041679	6800.307629	6038.800781	6830.942688
	YES	YES	YES	YES
PM LOS Performance Measures				
c_w capacity under prevailing conditions (veh/h)	6857	5675	6806	5634
v/c	0.679248752	0.416395327	0.691685086	0.424634594
S_w (mi/h)	60.4	60.7	60.3	60.7
S_{NW} (mi/h)	61.5	59.9	61.1	59.7
S (mi/h)	61.12182557	60.225916	60.824022	60.1092974
D (pc/mi/ln)	15.69762327	10.10335311	15.94382218	10.24867679
LOS	B	B	B	B

Dixon Ranch
3: Silva Valley Pkwy. & Green Valley Rd.

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	2	297	234	190	779	19	388	50	78	5	41	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6				4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Frt	1.00	1.00	0.85	1.00	1.00		1.00	0.91			0.98	
Flt Protected	0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (prot)	1770	1863	1583	1770	1856		1770	1692			1824	
Flt Permitted	0.95	1.00	1.00	0.95	1.00		0.95	1.00			1.00	
Satd. Flow (perm)	1770	1863	1583	1770	1856		1770	1692			1824	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	2	323	254	207	847	21	422	54	85	5	45	7
RTOR Reduction (vph)	0	0	169	0	1	0	0	45	0	0	5	0
Lane Group Flow (vph)	2	323	85	207	867	0	422	94	0	0	52	0
Turn Type	Prot	NA	Perm	Prot	NA		Split	NA		Split	NA	
Protected Phases	1	6		5	2		8	8		4	4	
Permitted Phases			6									
Actuated Green, G (s)	0.7	33.7	33.7	14.1	47.1		28.5	28.5			6.6	
Effective Green, g (s)	0.7	33.7	33.7	14.1	47.1		28.5	28.5			6.6	
Actuated g/C Ratio	0.01	0.33	0.33	0.14	0.47		0.28	0.28			0.07	
Clearance Time (s)	4.0	5.7	5.7	4.0	5.7		4.6	4.6			4.0	
Vehicle Extension (s)	2.5	3.0	3.0	2.5	3.0		2.5	2.5			2.5	
Lane Grp Cap (vph)	12	620	527	246	863		498	476			118	
v/s Ratio Prot	0.00	0.17		c0.12	c0.47		c0.24	0.06			c0.03	
v/s Ratio Perm			0.05									
w/c Ratio	0.17	0.52	0.16	0.84	1.01		0.85	0.20			0.44	
Uniform Delay, d1	50.0	27.2	23.8	42.5	27.1		34.3	27.7			45.5	
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00			1.00	
Incremental Delay, d2	4.7	0.8	0.1	21.8	31.9		12.4	0.1			1.9	
Delay (s)	54.7	28.0	23.9	64.2	59.0		46.7	27.8			47.5	
Level of Service	D	C	C	E	E		D	C			D	
Approach Delay (s)		26.3			60.0			42.0			47.5	
Approach LOS		C			E			D			D	
Intersection Summary												
HCM 2000 Control Delay			46.7									D
HCM 2000 Volume to Capacity ratio			0.92									
Actuated Cycle Length (s)			101.2					18.3				
Intersection Capacity Utilization			85.6%									E
Analysis Period (min)			15									
c Critical Lane Group												

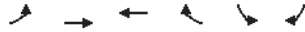
Dixon Ranch
4: Loch Way & Green Valley Rd

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (veh/h)	416	13	6	925	23	6
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	452	14	7	1005	25	7
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			466		1478	459
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			466		1478	459
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		82	99
cM capacity (veh/h)			1095		138	602
Direction, Lane #						
	EB 1	WB 1	NB 1	NB 2		
Volume Total	466	1012	25	7		
Volume Left	0	7	25	0		
Volume Right	14	0	0	7		
cSH	1700	1095	138	602		
Volume to Capacity	0.27	0.01	0.18	0.01		
Queue Length 95th (ft)	0	0	16	1		
Control Delay (s)	0.0	0.2	36.8	11.0		
Lane LOS		A	E	B		
Approach Delay (s)	0.0	0.2	31.5			
Approach LOS			D			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			63.5%			ICU Level of Service B
Analysis Period (min)			15			

Dixon Ranch
5: Green Valley Rd & Wilson Estates

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Volume (veh/h)	11	411	903	4	12	28
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	12	447	982	4	13	30
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	986			1454	984	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	986			1454	984	
tC, single (s)	4.1			6.4	6.2	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	98			91	90	
cM capacity (veh/h)	701			141	302	
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	459	986	43			
Volume Left	12	0	13			
Volume Right	0	4	30			
cSH	701	1700	225			
Volume to Capacity	0.02	0.58	0.19			
Queue Length 95th (ft)	1	0	17			
Control Delay (s)	0.5	0.0	24.8			
Lane LOS	A		C			
Approach Delay (s)	0.5	0.0	24.8			
Approach LOS			C			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization		57.8%		ICU Level of Service	B	
Analysis Period (min)		15				

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Volume (veh/h)	6	394	893	2	8	21
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	7	428	971	2	9	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	973			1413	972	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	973			1413	972	
tC, single (s)	4.1			6.4	6.2	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	99			94	93	
cM capacity (veh/h)	709			150	306	
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	435	973	32			
Volume Left	7	0	9			
Volume Right	0	2	23			
cSH	709	1700	238			
Volume to Capacity	0.01	0.57	0.13			
Queue Length 95th (ft)	1	0	11			
Control Delay (s)	0.3	0.0	22.4			
Lane LOS	A		C			
Approach Delay (s)	0.3	0.0	22.4			
Approach LOS			C			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization		57.1%		ICU Level of Service	B	
Analysis Period (min)		15				

Dixon Ranch
7: Deer Valley Rd. & Green Valley Rd.

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔			↔	
Volume (veh/h)	7	350	2	11	637	6	12	0	32	21	0	28
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	8	380	2	12	692	7	13	0	35	23	0	30
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type		None			None							
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	699			383			1143	1120	382	1150	1117	696
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	699			383			1143	1120	382	1150	1117	696
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			99			92	100	95	86	100	93
cM capacity (veh/h)	898			1176			163	203	666	164	203	442
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	8	383	12	699	48	53						
Volume Left	8	0	12	0	13	23						
Volume Right	0	2	0	7	35	30						
cSH	898	1700	1176	1700	361	256						
Volume to Capacity	0.01	0.23	0.01	0.41	0.13	0.21						
Queue Length 95th (ft)	1	0	1	0	11	19						
Control Delay (s)	9.0	0.0	8.1	0.0	16.5	22.7						
Lane LOS	A		A		C	C						
Approach Delay (s)	0.2		0.1		16.5	22.7						
Approach LOS					C	C						
Intersection Summary												
Average Delay				1.8								
Intersection Capacity Utilization			45.2%		ICU Level of Service		A					
Analysis Period (min)			15									

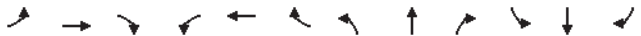
Dixon Ranch
8: Silver Springs Pkwy & Green Valley Rd

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔	↔	↔	↔
Volume (vph)	607	47	9	775	109	26
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0			4.0	4.0	
Lane Util. Factor	1.00			1.00	1.00	
Frt	0.99			1.00	0.97	
Flt Protected	1.00			1.00	0.96	
Satd. Flow (prot)	1845			1862	1744	
Flt Permitted	1.00			0.99	0.96	
Satd. Flow (perm)	1845			1849	1744	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	660	51	10	842	118	28
RTOR Reduction (vph)	5	0	0	0	15	0
Lane Group Flow (vph)	706	0	0	852	131	0
Turn Type	NA		Perm	NA	NA	
Protected Phases	4			8	2	
Permitted Phases			8			
Actuated Green, G (s)	25.2			25.2	9.0	
Effective Green, g (s)	25.2			25.2	9.0	
Actuated g/C Ratio	0.60			0.60	0.21	
Clearance Time (s)	4.0			4.0	4.0	
Vehicle Extension (s)	3.0			3.0	3.0	
Lane Grp Cap (vph)	1101			1104	371	
v/s Ratio Prot	0.38				c0.08	
v/s Ratio Perm				c0.46		
v/c Ratio	0.64			0.77	0.35	
Uniform Delay, d1	5.5			6.4	14.1	
Progression Factor	1.00			1.00	1.00	
Incremental Delay, d2	1.3			3.4	0.6	
Delay (s)	6.8			9.8	14.7	
Level of Service	A			A	B	
Approach Delay (s)	6.8			9.8	14.7	
Approach LOS	A			A	B	
Intersection Summary						
HCM 2000 Control Delay			9.0		HCM 2000 Level of Service	A
HCM 2000 Volume to Capacity ratio			0.66			
Actuated Cycle Length (s)			42.2		Sum of lost time (s)	8.0
Intersection Capacity Utilization			62.3%		ICU Level of Service	B
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch
11: Cameron Park Dr. & Green Valley Rd.


75-25 Modified Model, EPAP Plus Project
Timing Plan: AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	23	152	281	115	257	4	270	15	73	10	68	25
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Frt	1.00	0.90	1.00	1.00	1.00		1.00	0.88		1.00	0.96	
Flt Protected	0.95	1.00	0.95	1.00	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (prot)	1770	1681	1770	1859	1770		1770	1630		1770	1788	
Flt Permitted	0.95	1.00	0.95	1.00	1.00		0.95	1.00		0.95	1.00	
Satd. Flow (perm)	1770	1681	1770	1859	1770		1770	1630		1770	1788	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	25	165	305	125	279		4	293	16	79	11	74
RTOR Reduction (vph)	0	94	0	0	1		0	53	0	0	23	0
Lane Group Flow (vph)	25	376	0	125	282		0	293	42	0	11	78
Turn Type	Prot	NA	Prot	NA	Prot		NA	Prot	NA	Prot	NA	
Protected Phases	7	4	3	8	5		2	1	6			
Permitted Phases												
Actuated Green, G (s)	0.7	18.6	5.1	23.0	11.9		19.9	0.7	8.7			
Effective Green, g (s)	0.7	18.6	5.1	23.0	11.9		19.9	0.7	8.7			
Actuated g/C Ratio	0.01	0.31	0.08	0.38	0.20		0.33	0.01	0.14			
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	20	518	149	709	349		537	20	257			
v/s Ratio Prot	0.01	c0.22	c0.07	0.15	c0.17		0.03	0.01	c0.04			
v/s Ratio Perm												
w/c Ratio	1.25	0.73	0.84	0.40	0.84		0.08	0.55	0.30			
Uniform Delay, d1	29.8	18.6	27.2	13.6	23.3		13.9	29.6	23.1			
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00			
Incremental Delay, d2	288.2	5.0	31.8	0.4	16.1		0.1	28.9	0.7			
Delay (s)	318.0	23.6	59.0	14.0	39.4		14.0	58.5	23.8			
Level of Service	F	C	E	B	D		B	E	C			
Approach Delay (s)		38.5		27.8			33.1		27.2			
Approach LOS		D		C			C		C			
Intersection Summary												
HCM 2000 Control Delay		33.0							C			
HCM 2000 Volume to Capacity ratio		0.69										
Actuated Cycle Length (s)		60.3		Sum of lost time (s)			16.0					
Intersection Capacity Utilization		63.2%		ICU Level of Service			B					
Analysis Period (min)		15										
c Critical Lane Group												

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr.

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Sign Control	Stop			Stop			Stop			Stop		Stop
Volume (vph)	2	49	485	45	64	42	407	156	37	125	383	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	2	53	527	49	70	46	442	170	40	136	416	3
Direction, Lane #												
	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	55	527	164	442	210	136	420					
Volume Left (vph)	2	0	49	442	0	136	0					
Volume Right (vph)	0	527	46	0	40	0	3					
Hadj (s)	0.04	-0.57	-0.07	0.53	-0.10	0.53	0.03					
Departure Headway (s)	7.3	3.2	6.7	6.6	6.0	6.8	6.3					
Degree Utilization, x	0.11	0.47	0.31	0.81	0.35	0.26	0.73					
Capacity (veh/h)	447	1116	502	533	585	513	557					
Control Delay (s)	11.2	9.0	12.6	31.3	11.0	10.9	23.1					
Approach Delay (s)	9.2		12.6	24.8		20.1						
Approach LOS	A		B	C		C						
Intersection Summary												
Delay			17.8									
Level of Service			C									
Intersection Capacity Utilization			68.8%		ICU Level of Service		C					
Analysis Period (min)			15									

Dixon Ranch
20: Silva Valley Pkwy & WB US-50 Ramps

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations				↔	↔	↔	↔	↔			↔	↔
Volume (vph)	0	0	0	524	0	541	347	627	0	0	1091	558
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0			4.0	4.0
Lane Util. Factor				0.95	0.91	0.95	1.00	0.95			0.95	1.00
Frt				1.00	0.92	0.85	1.00	1.00			1.00	0.85
Flt Protected				0.95	0.98	1.00	0.95	1.00			1.00	1.00
Satd. Flow (prot)				1681	1519	1504	1770	3539			3539	1583
Flt Permitted				0.95	0.98	1.00	0.95	1.00			1.00	1.00
Satd. Flow (perm)				1681	1519	1504	1770	3539			3539	1583
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	0	0	570	0	588	377	682	0	0	1186	607
RTOR Reduction (vph)	0	0	0	0	58	200	0	0	0	0	0	391
Lane Group Flow (vph)	0	0	0	399	331	170	377	682	0	0	1186	216
Turn Type				Split	NA	Perm	Prot	NA			NA	Perm
Protected Phases				8	8		5	2			6	
Permitted Phases						8						6
Actuated Green, G (s)				20.9	20.9	20.9	18.6	51.1			28.5	28.5
Effective Green, g (s)				20.9	20.9	20.9	18.6	51.1			28.5	28.5
Actuated g/C Ratio				0.26	0.26	0.26	0.23	0.64			0.36	0.36
Clearance Time (s)				4.0	4.0	4.0	4.0	4.0			4.0	4.0
Vehicle Extension (s)				3.0	3.0	3.0	3.0	3.0			3.0	3.0
Lane Grp Cap (vph)				439	396	392	411	2260			1260	563
v/s Ratio Prot				c0.24	0.22		c0.21	0.19			c0.34	
v/s Ratio Perm						0.11						0.14
w/c Ratio				0.91	0.84	0.43	0.92	0.30			0.94	0.38
Uniform Delay, d1				28.6	27.9	24.6	29.9	6.5			24.9	19.2
Progression Factor				1.00	1.00	1.00	1.21	0.95			0.84	0.50
Incremental Delay, d2				22.3	14.2	0.8	23.7	0.3			12.5	1.6
Delay (s)				50.9	42.2	25.4	59.8	6.5			33.4	11.2
Level of Service				D	D	C	E	A			C	B
Approach Delay (s)	0.0				39.8			25.5			25.9	
Approach LOS	A				D			C			C	
Intersection Summary												
HCM 2000 Control Delay				29.8								
HCM 2000 Volume to Capacity ratio				0.92								
Actuated Cycle Length (s)				80.0					12.0			
Intersection Capacity Utilization				83.8%								
Analysis Period (min)				15								
c Critical Lane Group												

Dixon Ranch
21: Silva Valley Pkwy & County Club Dr.

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (vph)	163	157	1006	161	1	1486
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	0.95	1.00	1.00	0.95
Frt	1.00	0.85	0.98	1.00	1.00	1.00
Flt Protected	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (prot)	1770	1583	3466	1770	1770	3539
Flt Permitted	0.95	1.00	1.00	0.95	1.00	1.00
Satd. Flow (perm)	1770	1583	3466	1770	1770	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	177	171	1093	175	1	1615
RTOR Reduction (vph)	0	140	12	0	0	0
Lane Group Flow (vph)	177	31	1256	0	1	1615
Turn Type	NA	Perm	NA		Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8				
Actuated Green, G (s)	13.2	13.2	54.0		0.8	58.8
Effective Green, g (s)	13.2	13.2	54.0		0.8	58.8
Actuated g/C Ratio	0.16	0.16	0.68		0.01	0.73
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	292	261	2339		17	2601
v/s Ratio Prot	c0.10		0.36		0.00	c0.46
v/s Ratio Perm		0.02				
w/c Ratio	0.61	0.12	0.54		0.06	0.62
Uniform Delay, d1	31.0	28.4	6.6		39.2	5.2
Progression Factor	1.00	1.00	1.13		1.00	1.00
Incremental Delay, d2	3.5	0.2	0.8		1.5	1.1
Delay (s)	34.5	28.6	8.3		40.7	6.3
Level of Service	C	C	A		D	A
Approach Delay (s)	31.6		8.3			6.3
Approach LOS	C		A			A
Intersection Summary						
HCM 2000 Control Delay			9.8			
HCM 2000 Volume to Capacity ratio			0.65			
Actuated Cycle Length (s)			80.0			12.0
Intersection Capacity Utilization			56.8%			
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch
24: Silva Valley Pkwy. & Appian Way

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	37	1	87	155	2	63	22	242	46	26	337	21
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	40	1	95	168	2	68	24	263	50	28	366	23
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	136	239	337	417								
Volume Left (vph)	40	168	24	28								
Volume Right (vph)	95	68	50	23								
Hadj (s)	-0.32	0.00	-0.04	0.01								
Departure Headway (s)	6.4	6.4	5.9	5.8								
Degree Utilization, x	0.24	0.43	0.55	0.67								
Capacity (veh/h)	460	490	569	584								
Control Delay (s)	11.5	14.2	15.9	19.9								
Approach Delay (s)	11.5	14.2	15.9	19.9								
Approach LOS	B	B	C	C								
Intersection Summary												
Delay	16.5											
Level of Service	C											
Intersection Capacity Utilization	52.1%		ICU Level of Service		A							
Analysis Period (min)	15											

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.

75-25 Modified Model, EPAP Plus Project
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↕			↕		↕
Volume (veh/h)	352	39	0	867	0	21
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	383	42	0	942	0	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)	766					
pX, platoon unblocked					0.67	
vC, conflicting volume				425	1346	404
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				425	1270	404
tC, single (s)				4.1	6.4	6.2
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	96
cM capacity (veh/h)				1134	124	647
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	425	942	23			
Volume Left	0	0	0			
Volume Right	42	0	23			
cSH	1700	1700	647			
Volume to Capacity	0.25	0.55	0.04			
Queue Length 95th (ft)	0	0	3			
Control Delay (s)	0.0	0.0	10.8			
Lane LOS				B		
Approach Delay (s)	0.0	0.0	10.8			
Approach LOS				B		
Intersection Summary						
Average Delay				0.2		
Intersection Capacity Utilization	49.0%		ICU Level of Service		A	
Analysis Period (min)	15					

Dixon Ranch
4: Loch Way & Green Valley Rd

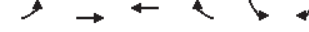
75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (veh/h)	1044	27	4	592	21	4
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	1135	29	4	643	23	4
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			1164		1802	1149
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			1164		1802	1149
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			99		74	98
cM capacity (veh/h)			600		87	241
Direction, Lane #	EB 1	WB 1	NB 1	NB 2		
Volume Total	1164	648	23	4		
Volume Left	0	4	23	0		
Volume Right	29	0	0	4		
cSH	1700	600	87	241		
Volume to Capacity	0.68	0.01	0.26	0.02		
Queue Length 95th (ft)	0	1	24	1		
Control Delay (s)	0.0	0.2	60.6	20.2		
Lane LOS		A	F	C		
Approach Delay (s)	0.0	0.2	54.1			
Approach LOS			F			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization		66.6%		ICU Level of Service	C	
Analysis Period (min)		15				

Dixon Ranch
5: Green Valley Rd & Wilson Estates

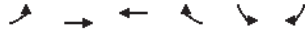
75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	31	1017	577	12	8	19
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	34	1105	627	13	9	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	640			1807	634	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	640			1807	634	
tC, single (s)	4.1			6.4	6.2	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	96			90	96	
cM capacity (veh/h)	944			84	479	
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	1139	640	29			
Volume Left	34	0	9			
Volume Right	0	13	21			
cSH	944	1700	200			
Volume to Capacity	0.04	0.38	0.15			
Queue Length 95th (ft)	3	0	13			
Control Delay (s)	1.2	0.0	26.1			
Lane LOS	A		D			
Approach Delay (s)	1.2	0.0	26.1			
Approach LOS			D			
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilization		88.5%		ICU Level of Service	E	
Analysis Period (min)		15				

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd

75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Volume (veh/h)	12	1041	588	5	10	14
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	13	1132	639	5	11	15
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None	None				
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	645			1799	642	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	645			1799	642	
tC, single (s)	4.1			6.4	6.2	
tC, 2 stage (s)						
tF (s)	2.2			3.5	3.3	
p0 queue free %	99			87	97	
cM capacity (veh/h)	941			87	474	
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	1145	645	26			
Volume Left	13	0	11			
Volume Right	0	5	15			
cSH	941	1700	166			
Volume to Capacity	0.01	0.38	0.16			
Queue Length 95th (ft)	1	0	14			
Control Delay (s)	0.5	0.0	30.8			
Lane LOS	A		D			
Approach Delay (s)	0.5	0.0	30.8			
Approach LOS			D			
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization			74.4%	ICU Level of Service	D	
Analysis Period (min)			15			

Dixon Ranch
7: Deer Valley Rd. & Green Valley Rd.


75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↔	↔		↔	↔
Volume (veh/h)	45	771	18	39	486	7	8	1	23	7	0	14
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	49	838	20	42	528	8	9	1	25	8	0	15
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None			None								
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	536			858			1574	1566	848	1578	1572	532
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	536			858			1574	1566	848	1578	1572	532
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	95			95			89	99	93	90	100	97
cM capacity (veh/h)	1032			783			80	100	361	76	99	547
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	49	858	42	536	35	23						
Volume Left	49	0	42	0	9	8						
Volume Right	0	20	0	8	25	15						
cSH	1032	1700	783	1700	184	178						
Volume to Capacity	0.05	0.50	0.05	0.32	0.19	0.13						
Queue Length 95th (ft)	4	0	4	0	17	11						
Control Delay (s)	8.7	0.0	9.9	0.0	29.0	28.2						
Lane LOS	A		A		D	D						
Approach Delay (s)	0.5			0.7			29.0			28.2		
Approach LOS							D			D		
Intersection Summary												
Average Delay					1.6							
Intersection Capacity Utilization					51.7%	ICU Level of Service	A					
Analysis Period (min)					15							

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr.


75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↕		↕	↕	↕	↕		↕	↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	0	41	497	26	36	40	535	401	19	9	220	2
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	45	540	28	39	43	582	436	21	10	239	2
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	45	540	111	582	457	10	241					
Volume Left (vph)	0	0	28	582	0	10	0					
Volume Right (vph)	0	540	43	0	21	0	2					
Hadj (s)	0.03	-0.57	-0.15	0.53	0.00	0.53	0.03					
Departure Headway (s)	6.6	3.2	6.2	5.9	5.3	6.9	6.4					
Degree Utilization, x	0.08	0.48	0.19	0.95	0.68	0.02	0.43					
Capacity (veh/h)	514	1116	556	605	658	507	553					
Control Delay (s)	10.2	9.1	10.7	47.8	17.6	8.8	12.8					
Approach Delay (s)	9.2		10.7	34.5		12.6						
Approach LOS	A		B	D		B						
Intersection Summary												
Delay			23.0									
Level of Service			C									
Intersection Capacity Utilization			63.8%		ICU Level of Service		B					
Analysis Period (min)			15									

Dixon Ranch
13: El Dorado Hills Blvd. & Harvard Way

75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM



Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	↕	↕	↕	↕	↕	↕
Volume (vph)	141	125	969	184	162	603
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0		4.0	4.0
Lane Util. Factor	1.00	1.00	0.95		0.97	0.95
Frt	1.00	0.85	0.98		1.00	1.00
Flt Protected	0.95	1.00	1.00		0.95	1.00
Satd. Flow (prot)	1770	1583	3454		3433	3539
Flt Permitted	0.95	1.00	1.00		0.95	1.00
Satd. Flow (perm)	1770	1583	3454		3433	3539
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	153	136	1053	200	176	655
RTOR Reduction (vph)	0	115	22	0	0	0
Lane Group Flow (vph)	153	21	1231	0	176	655
Turn Type	NA	Perm	NA		Prot	NA
Protected Phases	8		2		1	6
Permitted Phases		8				
Actuated Green, G (s)	7.9	7.9	27.5		3.7	35.2
Effective Green, g (s)	7.9	7.9	27.5		3.7	35.2
Actuated g/C Ratio	0.15	0.15	0.54		0.07	0.69
Clearance Time (s)	4.0	4.0	4.0		4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0		3.0	3.0
Lane Grp Cap (vph)	273	244	1858		248	2437
v/s Ratio Prot	c0.09		c0.36		c0.05	0.19
v/s Ratio Perm		0.01				
w/c Ratio	0.56	0.09	0.66		0.71	0.27
Uniform Delay, d1	20.0	18.5	8.5		23.2	3.0
Progression Factor	1.00	1.00	1.00		1.00	1.00
Incremental Delay, d2	2.6	0.2	0.9		9.0	0.1
Delay (s)	22.6	18.7	9.4		32.1	3.1
Level of Service	C	B	A		C	A
Approach Delay (s)	20.8		9.4		9.2	
Approach LOS	C		A		A	
Intersection Summary						
HCM 2000 Control Delay			10.7	HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio			0.65			
Actuated Cycle Length (s)			51.1	Sum of lost time (s)		12.0
Intersection Capacity Utilization			55.1%	ICU Level of Service		B
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.

75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↑			↑		↑
Volume (veh/h)	895	117	0	574	0	14
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	973	127	0	624	0	15
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)			716			
pX, platoon unblocked				0.89		
vC, conflicting volume			1100	1660	1036	
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol		1100		1679	1036	
tC, single (s)		4.1		6.4	6.2	
tC, 2 stage (s)						
tF (s)		2.2		3.5	3.3	
p0 queue free %		100		100	95	
cM capacity (veh/h)		635		93	281	
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	1100	624	15			
Volume Left	0	0	0			
Volume Right	127	0	15			
cSH	1700	1700	281			
Volume to Capacity	0.65	0.37	0.05			
Queue Length 95th (ft)	0	0	4			
Control Delay (s)	0.0	0.0	18.5			
Lane LOS			C			
Approach Delay (s)	0.0	0.0	18.5			
Approach LOS			C			
Intersection Summary						
Average Delay		0.2				
Intersection Capacity Utilization		64.2%		ICU Level of Service		C
Analysis Period (min)		15				

Dixon Ranch
26: Site Dwy. Full/Site Dwy. & Green Valley Rd.

75-25 Mitigated Model, EPAP Plus Project
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑			↑	↑		↑	↑
Volume (vph)	0	792	117	70	435	0	139	0	28	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)		4.0		4.0	4.0			4.0	4.0			
Lane Util. Factor		1.00		1.00	1.00			1.00	1.00			
Frt		0.98		1.00	1.00			1.00	0.85			
Flt Protected		1.00		0.95	1.00			0.95	1.00			
Satd. Flow (prot)		1827		1770	1863			1770	1583			
Flt Permitted		1.00		0.15	1.00			0.76	1.00			
Satd. Flow (perm)		1827		276	1863			1410	1583			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	861	127	76	473	0	151	0	30	0	0	0
RTOR Reduction (vph)	0	8	0	0	0	0	0	0	25	0	0	0
Lane Group Flow (vph)	0	980	0	76	473	0	0	151	5	0	0	0
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm		
Protected Phases		6			2			4			8	
Permitted Phases	6			2			4		4	8		
Actuated Green, G (s)		31.1		31.1	31.1			8.4	8.4			
Effective Green, g (s)		31.1		31.1	31.1			8.4	8.4			
Actuated g/C Ratio		0.65		0.65	0.65			0.18	0.18			
Clearance Time (s)		4.0		4.0	4.0			4.0	4.0			
Vehicle Extension (s)		3.0		3.0	3.0			3.0	3.0			
Lane Grp Cap (vph)		1196		180	1219			249	279			
v/s Ratio Prot		c0.54			0.25							
v/s Ratio Perm				0.28				c0.11	0.00			
w/c Ratio		0.82		0.42	0.39			0.61	0.02			
Uniform Delay, d1		6.1		3.9	3.8			18.0	16.1			
Progression Factor		1.00		1.00	1.00			1.00	1.00			
Incremental Delay, d2		4.5		1.6	0.2			4.1	0.0			
Delay (s)		10.6		5.5	4.0			22.2	16.2			
Level of Service		B		A	A			C	B			
Approach Delay (s)		10.6			4.2			21.2				0.0
Approach LOS		B			A			C				A
Intersection Summary												
HCM 2000 Control Delay		9.7										A
HCM 2000 Volume to Capacity ratio		0.77										
Actuated Cycle Length (s)		47.5			Sum of lost time (s)				8.0			
Intersection Capacity Utilization		70.4%			ICU Level of Service				C			
Analysis Period (min)		15										
c Critical Lane Group												

Basic

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				Existing plus Approved Projects plus Proposed Project Conditions														
				Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures							
	Length (ft)	Number of Lanes (N)	Interchange Density (I/mi)	AM Peak	PM Peak	V _b	FFS	S	D	LOS	V _b	FFS	S	D	LOS			
				(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)					
Eastbound	West of El Dorado Hills Blvd	8180	4	0.67	3,111	5,685	870.742	73.11	75	74.815	11.639	B	1591.182	73.11	75	71.1311	22.37	C
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	1490	4	0.67	1,161	3,185	324.954	73.11	75	69.9555	4.6451	A	891.4538	73.11	75	74.8696	11.907	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy SB On Ramp	1130	4	0.50	1,517	3,619	424.595	73.6	75	71.3348	5.9521	A	1012.927	73.6	75	74.9982	13.506	B
	East of Silva Valley Pkwy	1980	4	0.50	2,700	5,228	755.707	73.6	75	74.3394	10.166	A	1463.272	73.6	75	72.6241	20.149	C
Westbound	East of Silva Valley Pkwy	1950	3	0.50	5,143	2,977	1919.31	73.6	75	65.6444	29.238	D	1110.982	73.6	75	74.8637	14.84	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy NB On Ramp	1640	3	0.50	4,078	1,507	1521.86	73.6	75	71.9852	21.141	C	562.3949	73.6	75	72.8801	7.7167	A
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	3080	3	0.67	3,985	1,617	1487.16	73.11	75	72.3729	20.549	C	603.4457	73.11	75	73.2592	8.2371	A
	West of El Dorado Hills Blvd	6320	3	0.67	5,838	3,207	2178.67	73.11	75	59.6208	36.542	E	1196.815	73.11	75	74.5712	16.049	B

Universal Inputs:
 PHF 0.92
 (P_v) 6%
 f_{hv} 0.970873786

Merge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

		Segment Inputs					AM Flow Inputs			AM LOS Performance Measures										Existing plus Approved Project p	
		Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
		(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/in)						(pc/mi/in)		
EB	Silva Valley Pkwy SB On Ramp	4	1	200	2700	1517	1183	3023	1698	1324	49	0.116	196.94	9600	751	148	679	0.3149	19.241	B	
	Silva Valley Pkwy NB On Ramp	4	1	1000	2700	2700	0	3023	3023	0	86	0.2178	658.37	9600	1182	494	1209	0.3149	8.6362	A	
WB	El Dorado Hills Blvd On Ramp	3	1	800	5838	3985	1853	6536	4461.5	2074.6	127	0.5999	2676.4	7200	1785	2007	2676	0.9078	36.562	E	
Universal Inputs:																					
Length		1500 (ft)																			
S _{FF}		70 (mi/h)																			
S _{FR}		35 (mi/h)																			
PHF		0.92																			
[P _c]		6%																			
f _{inv}		0.970873786																			

Merge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs			Plus Proposed Project Conditions																	
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _a)	PM Flow Inputs			PM LOS Performance Measures													
				Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	V _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/hr)	(pc/h)			(pc/h/in)						(pc/mi/in)		
EB	Silva Valley Pkwy SB On Ramp	4	1	200	5228	3619	1609	5853	4052	1801	116	-0.0074	-29.871	9600	2041	-22	1621	0.6097	30.084	D
	Silva Valley Pkwy NB On Ramp	4	1	1000	5228	5228	0	5853	5853	0	167	0.2178	1274.8	9600	2289	956	2341	0.6097	17.467	B
WB	El Dorado Hills Blvd On Ramp	3	1	800	3207	1617	1590	3590.4	1810.3	1780.1	52	0.5999	1086	7200	724	815	1086	0.4987	21.996	C
Universal Inputs:																				
	Length	1500	(ft)																	
	S _{FR}	70	(mi/h)																	
	S _{FR}	35	(mi/h)																	
	PHF	0.92																		
	(P _r)	6%																		
	f _{ov}	0.970873786																		

Diverge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				AM Flow Inputs			AM LOS Performance Measures												
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume	Upstream Volume	Ramp Volume	V _D	V _F	V _R	P _{FD}	V ₁₂	Capacity	V ₃	V _{12a}	v/c	D	LOS		
							(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)				(pc/mi/ln)	
EB	Latrobe SB Off Ramp	4	1	140	1629	3111	1482	1823.77	3483	1659.2	0.436	2454.4	9600	514	1841	2454	0.3628	24.099	C
	Latrobe NB Off Ramp	4	1	120	1161	1629	468	1299.82	1823.8	523.96	0.436	1090.7	9600	367	818	1091	0.19	12.552	B
WB	Silva Valley Pkwy Off Ramp	3	2	1300	4078	5143	1065	4565.59	5757.9	1192.3	0.5612	3754.6	7200	2003	2816	3755	0.7997	24.841	C

Universal Inputs:
 Length 1500 (ft)
 S_{FF} 70 (mi/h)
 S_{FR} 35 (mi/h)
 PHF 0.92
 P₁ 6%
 F_{RV} 0.970873786

Diverge

75-25 Mod-Existing Basic Freeway Segments (State Route 50)

Segment Inputs				is Proposed Project Conditions															
Segment Inputs				PM Flow Inputs			PM LOS Performance Measures												
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	V _D	V _F	V _R	P _{FD}	V ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)					(pc/mi/ln)			
EB	Latrobe SB Off Ramp	4	1	140	4262	5685	1423	4771.59	6364.7	1593.1	0.436	3673.6	9600	1346	2755	3674	0.663	34.585	D
	Latrobe NB Off Ramp	4	1	120	3185	4262	1077	3565.82	4771.6	1205.8	0.436	2760.5	9600	1006	2070	2760	0.497	26.912	C
WB	Silva Valley Pkwy Off Ramp	3	2	1300	1507	2977	1470	1687.18	3332.9	1645.8	0.601	2659.7	7200	673	1995	2660	0.4629	15.426	B
Universal Inputs: Leng: 1500 (ft) S _{FF} : 70 (mi/h) S _{FR} : 35 (mi/h) PHF: 0.92 P ₁ : 6% F _{HW} : 0.970873786																			

Attachment C
Cumulative (2035) Analysis Worksheets

Dixon Ranch
5: Green Valley Rd & Wilson Estates

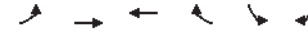
75-25 Modified Model, Cumulative
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	10	279	544	5	12	27
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	303	591	5	13	29
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	597				919	594
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	597				919	594
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				96	94
cM capacity (veh/h)	980				298	505
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	314	597	42			
Volume Left	11	0	13			
Volume Right	0	5	29			
cSH	980	1700	416			
Volume to Capacity	0.01	0.35	0.10			
Queue Length 95th (ft)	1	0	8			
Control Delay (s)	0.4	0.0	14.6			
Lane LOS	A		B			
Approach Delay (s)	0.4	0.0	14.6			
Approach LOS			B			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			38.9%	ICU Level of Service	A	
Analysis Period (min)			15			

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd


75-25 Modified Model, Cumulative
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	4	260	510	5	10	25
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	283	554	5	11	27
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	560				848	557
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	560				848	557
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	100				97	95
cM capacity (veh/h)	1011				330	530
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	287	560	38			
Volume Left	4	0	11			
Volume Right	0	5	27			
cSH	1011	1700	452			
Volume to Capacity	0.00	0.33	0.08			
Queue Length 95th (ft)	0	0	7			
Control Delay (s)	0.2	0.0	13.7			
Lane LOS	A		B			
Approach Delay (s)	0.2	0.0	13.7			
Approach LOS			B			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization			37.1%	ICU Level of Service	A	
Analysis Period (min)			15			

Dixon Ranch
24: Silva Valley Pkwy. & Appian Way


75-25 Modified Model, Cumulative
Timing Plan: AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	39	1	92	158	2	63	25	242	52	30	293	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	42	1	100	172	2	68	27	263	57	33	318	27
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	143	242	347	378								
Volume Left (vph)	42	172	27	33								
Volume Right (vph)	100	68	57	27								
Hadj (s)	-0.33	0.01	-0.05	0.01								
Departure Headway (s)	6.3	6.4	5.8	5.8								
Degree Utilization, x	0.25	0.43	0.56	0.61								
Capacity (veh/h)	473	496	576	575								
Control Delay (s)	11.4	14.1	16.1	17.7								
Approach Delay (s)	11.4	14.1	16.1	17.7								
Approach LOS	B	B	C	C								
Intersection Summary												
Delay	15.6											
Level of Service	C											
Intersection Capacity Utilization	51.0%		ICU Level of Service	A								
Analysis Period (min)	15											

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.


75-25 Modified Model, Cumulative
Timing Plan: AM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↕			↕		↕
Volume (veh/h)	256	0	0	543	0	0
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	278	0	0	590	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	796					
pX, platoon unblocked					0.77	
vC, conflicting volume				278	868	278
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				278	677	278
tC, single (s)				4.1	6.4	6.2
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	100
cM capacity (veh/h)				1284	321	761
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total (vph)	278	590	0			
Volume Left (vph)	0	0	0			
Volume Right (vph)	0	0	0			
cSH	1700	1700	1700			
Volume to Capacity	0.16	0.35	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS				A		
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS				A		
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	31.9%		ICU Level of Service	A		
Analysis Period (min)	15					

Dixon Ranch
4: Loch Way & Green Valley Rd


75-25 Modified Model, Cumulative
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔	↔	↔	↔	↔	↔
Volume (veh/h)	537	177	83	268	132	83
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	584	192	90	291	143	90
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			776		1152	680
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			776		1152	680
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			89		26	80
cM capacity (veh/h)			840		195	451
Direction, Lane #	EB 1	WB 1	NB 1	NB 2		
Volume Total	776	382	143	90		
Volume Left	0	90	143	0		
Volume Right	192	0	0	90		
cSH	1700	840	195	451		
Volume to Capacity	0.46	0.11	0.74	0.20		
Queue Length 95th (ft)	0	9	120	18		
Control Delay (s)	0.0	3.3	62.1	15.0		
Lane LOS		A	F	B		
Approach Delay (s)	0.0	3.3	43.9			
Approach LOS			E			
Intersection Summary						
Average Delay			8.3			
Intersection Capacity Utilization			75.0%	ICU Level of Service	D	
Analysis Period (min)			15			

Dixon Ranch
5: Green Valley Rd & Wilson Estates

75-25 Modified Model, Cumulative
Timing Plan: PM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	↔
Volume (veh/h)	27	621	343	16	8	19
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	29	675	373	17	9	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	390				1115	382
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	390				1115	382
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				96	97
cM capacity (veh/h)	1168				224	666
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	704	390	29			
Volume Left	29	0	9			
Volume Right	0	17	21			
cSH	1168	1700	420			
Volume to Capacity	0.03	0.23	0.07			
Queue Length 95th (ft)	2	0	6			
Control Delay (s)	0.7	0.0	14.2			
Lane LOS	A		B			
Approach Delay (s)	0.7	0.0	14.2			
Approach LOS			B			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization			64.6%	ICU Level of Service	C	
Analysis Period (min)			15			

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr.

75-25 Modified Model, Cumulative
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↕		↕	↕	↕	↕		↕	↕	↕
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	0	33	536	31	30	47	552	548	30	11	303	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	36	583	34	33	51	600	596	33	12	329	3
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	36	583	117	600	628	12	333					
Volume Left (vph)	0	0	34	600	0	12	0					
Volume Right (vph)	0	583	51	0	33	0	3					
Hadj (s)	0.03	-0.57	-0.17	0.53	0.00	0.53	0.03					
Departure Headway (s)	6.9	3.2	6.4	6.0	5.5	7.1	6.6					
Degree Utilization, x	0.07	0.52	0.21	1.01	0.96	0.02	0.61					
Capacity (veh/h)	484	1117	535	600	648	492	526					
Control Delay (s)	10.4	9.6	11.1	61.5	47.6	9.1	18.2					
Approach Delay (s)	9.6		11.1	54.4		17.9						
Approach LOS	A		B	F		C						
Intersection Summary												
Delay			34.8									
Level of Service			D									
Intersection Capacity Utilization			69.5%		ICU Level of Service		C					
Analysis Period (min)			15									

Dixon Ranch
13: El Dorado Hills Blvd. & Harvard Way

75-25 Modified Model, Cumulative
Timing Plan: PM

Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	↕	↕	↕	↕	↕	↕	
Volume (vph)	353	100	1070	461	123	684	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	1.00	0.95	1.00	0.97	0.95	
Frt	1.00	0.85	1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1583	3539	1583	3433	3539	
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1583	3539	1583	3433	3539	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	384	109	1163	501	134	743	
RTOR Reduction (vph)	0	78	0	280	0	0	
Lane Group Flow (vph)	384	31	1163	221	134	743	
Turn Type	NA	Perm	NA	Perm	Prot	NA	
Protected Phases	8		2		1	6	
Permitted Phases		8		2			
Actuated Green, G (s)	15.7	15.7	24.2	24.2	2.9	31.1	
Effective Green, g (s)	15.7	15.7	24.2	24.2	2.9	31.1	
Actuated g/C Ratio	0.29	0.29	0.44	0.44	0.05	0.57	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	507	453	1562	699	181	2008	
v/s Ratio Prot	c0.22		c0.33		c0.04	0.21	
v/s Ratio Perm		0.02		0.14			
w/c Ratio	0.76	0.07	0.74	0.32	0.74	0.37	
Uniform Delay, d1	17.8	14.2	12.7	9.9	25.6	6.5	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	6.4	0.1	2.0	0.3	15.0	0.1	
Delay (s)	24.2	14.3	14.7	10.2	40.6	6.6	
Level of Service	C	B	B	B	D	A	
Approach Delay (s)	22.0		13.3			11.8	
Approach LOS	C		B			B	
Intersection Summary							
HCM 2000 Control Delay			14.3		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio			0.75				
Actuated Cycle Length (s)			54.8		Sum of lost time (s)		12.0
Intersection Capacity Utilization			62.6%		ICU Level of Service		B
Analysis Period (min)			15				

c Critical Lane Group

Dixon Ranch
23: Harvard Way & Silva Valley Pkwy.

75-25 Modified Model, Cumulative
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↑	↘	↔	↑	↘	↔	↑	↘	↔	↑	↘
Volume (vph)	173	14	264	10	12	6	223	358	13	12	252	86
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	1.00	0.85	1.00	0.95	1.00	0.99	1.00	1.00	0.85	1.00	0.85
Flt Protected	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00
Satd. Flow (prot)	1770	1863	1583	1770	1765	1770	1853	1770	1863	1583	1770	1863
Flt Permitted	0.95	1.00	1.00	0.95	1.00	0.95	1.00	0.95	1.00	1.00	1.00	1.00
Satd. Flow (perm)	1770	1863	1583	1770	1765	1770	1853	1770	1863	1583	1770	1863
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	188	15	287	11	13	7	242	389	14	13	274	93
RTOR Reduction (vph)	0	0	239	0	7	0	0	2	0	0	0	65
Lane Group Flow (vph)	188	15	48	11	13	0	242	401	0	13	274	28
Turn Type	Prot	NA	Perm	Prot	NA	Prot	NA	Prot	NA	Perm	Prot	NA
Protected Phases	7	4		3	8	5	2	1	6		6	
Permitted Phases			4									6
Actuated Green, G (s)	7.3	8.1	8.1	0.6	1.4	9.3	23.5	0.6	14.8	14.8	7.3	8.1
Effective Green, g (s)	7.3	8.1	8.1	0.6	1.4	9.3	23.5	0.6	14.8	14.8	7.3	8.1
Actuated g/C Ratio	0.15	0.17	0.17	0.01	0.03	0.19	0.48	0.01	0.30	0.30	0.15	0.17
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
Lane Grp Cap (vph)	264	309	262	21	50	337	892	21	565	480	264	309
v/s Ratio Prot	c0.11	0.01		0.01	0.01	c0.14	c0.22	0.01	0.15		c0.11	0.01
v/s Ratio Perm			c0.03									c0.03
w/c Ratio	0.71	0.05	0.18	0.52	0.26	0.72	0.45	0.62	0.48	0.06	0.71	0.05
Uniform Delay, d1	19.7	17.1	17.5	24.0	23.2	18.5	8.4	24.0	13.9	12.1	19.7	17.1
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Incremental Delay, d2	8.8	0.1	0.3	21.6	2.8	7.1	0.4	43.9	0.7	0.1	8.8	0.1
Delay (s)	28.5	17.2	17.8	45.5	26.0	25.7	8.7	67.9	14.5	12.1	28.5	17.2
Level of Service	C	B	B	D	C	C	A	E	B	B	C	B
Approach Delay (s)		21.9			32.9		15.1		15.8			21.9
Approach LOS		C			C		B		B			C
Intersection Summary												
HCM 2000 Control Delay	17.8			HCM 2000 Level of Service				B				
HCM 2000 Volume to Capacity ratio	0.62											
Actuated Cycle Length (s)	48.8			Sum of lost time (s)				16.0				
Intersection Capacity Utilization	51.9%			ICU Level of Service				A				
Analysis Period (min)	15											
c Critical Lane Group												

Dixon Ranch
24: Silva Valley Pkwy. & Appian Way

75-25 Modified Model, Cumulative
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↑	↘	↔	↑	↘	↔	↑	↘	↔	↑	↘
Sign Control	Stop			Stop			Stop			Stop		
Volume (vph)	19	4	43	57	2	44	89	309	113	61	247	38
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	21	4	47	62	2	48	97	336	123	66	268	41
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	72	112	555	376								
Volume Left (vph)	21	62	97	66								
Volume Right (vph)	47	48	123	41								
Hadj (s)	-0.30	-0.11	-0.06	0.00								
Departure Headway (s)	6.2	6.3	5.0	5.3								
Degree Utilization, x	0.12	0.19	0.77	0.55								
Capacity (veh/h)	505	508	707	658								
Control Delay (s)	10.1	10.8	22.3	14.5								
Approach Delay (s)	10.1	10.8	22.3	14.5								
Approach LOS	B	B	C	B								
Intersection Summary												
Delay	17.7											
Level of Service	C											
Intersection Capacity Utilization	55.1%			ICU Level of Service				B				
Analysis Period (min)	15											

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.

75-25 Modified Model, Cumulative
Timing Plan: PM

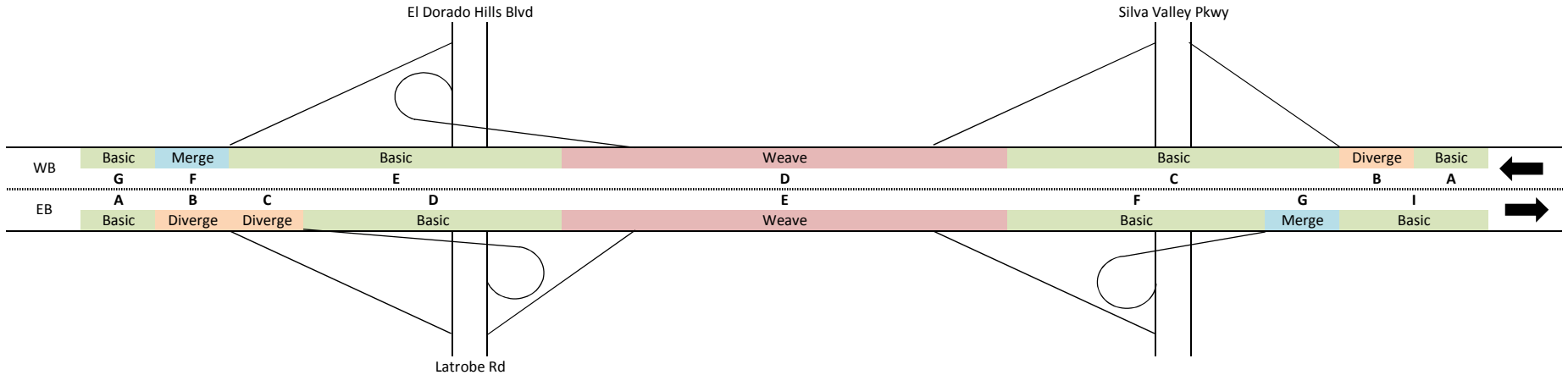
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Volume (veh/h)	612	0	0	367	0	0
Sign Control	Free		Free		Stop	
Grade	0%					
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	665	0	0	399	0	0
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None		None			
Median storage (veh)						
Upstream signal (ft)	836					
pX, platoon unblocked				0.96		
vC, conflicting volume	665			1064		665
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	665			1046		665
tC, single (s)	4.1		6.4		6.2	
tC, 2 stage (s)						
tF (s)	2.2		3.5		3.3	
p0 queue free %	100		100		100	
cM capacity (veh/h)	924		243		460	
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	665	399	0			
Volume Left	0	0	0			
Volume Right	0	0	0			
cSH	1700	1700	1700			
Volume to Capacity	0.39	0.23	0.00			
Queue Length 95th (ft)	0	0	0			
Control Delay (s)	0.0	0.0	0.0			
Lane LOS	A		A			
Approach Delay (s)	0.0	0.0	0.0			
Approach LOS	A		A			
Intersection Summary						
Average Delay	0.0					
Intersection Capacity Utilization	35.5%		ICU Level of Service		A	
Analysis Period (min)	15					

Dixon Ranch
26: Site Dwy, Full/Site Dwy. & Green Valley Rd.

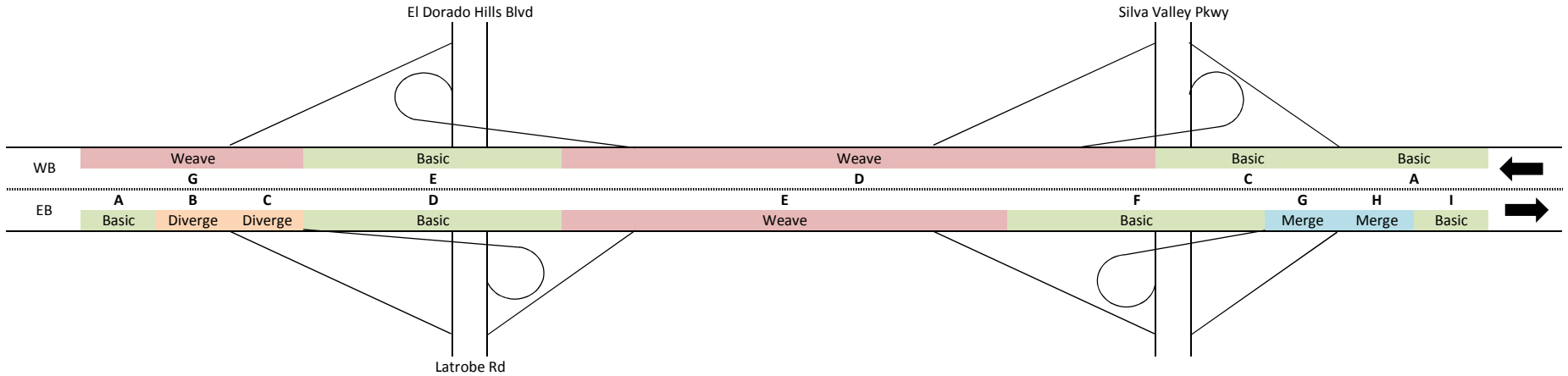
75-25 Modified Model, Cumulative
Timing Plan: PM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Volume (vph)	0	612	0	0	367	0	0	0	0	0	0	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0											
Lane Util. Factor	1.00			1.00								
Frt	1.00											
FIt Protected	1.00			1.00								
Satd. Flow (prot)	1863			1863								
FIt Permitted	1.00											
Satd. Flow (perm)	1863			1863								
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	0	665	0	0	399	0	0	0	0	0	0	0
RTOR Reduction (vph)	0	0	0	0	0	0	0	0	0	0	0	0
Lane Group Flow (vph)	0	665	0	0	399	0	0	0	0	0	0	0
Turn Type	Perm		NA		Perm		NA		Perm		Perm	
Protected Phases	6		2		4		8					
Permitted Phases	6											
Actuated Green, G (s)	15.4			15.4								
Effective Green, g (s)	15.4			15.4								
Actuated g/C Ratio	0.53			0.53								
Clearance Time (s)	4.0			4.0								
Vehicle Extension (s)	3.0			3.0								
Lane Grp Cap (vph)	985			985								
v/s Ratio Prot	c0.36			0.21								
v/s Ratio Perm												
w/c Ratio	0.68			0.41								
Uniform Delay, d1	5.0			4.1								
Progression Factor	1.00			1.00								
Incremental Delay, d2	1.8			0.3								
Delay (s)	6.9			4.4								
Level of Service	A			A								
Approach Delay (s)	6.9			4.4			0.0		0.0			
Approach LOS	A			A			A		A			
Intersection Summary												
HCM 2000 Control Delay	5.9			HCM 2000 Level of Service			A					
HCM 2000 Volume to Capacity ratio	0.49											
Actuated Cycle Length (s)	29.1			Sum of lost time (s)			8.0					
Intersection Capacity Utilization	35.5%			ICU Level of Service			A					
Analysis Period (min)	15											
c Critical Lane Group												

Existing



Cumulative



Basic

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

Segment Inputs				Cumulative Conditions												Cumulative plus Proposed Project Conditions																
				Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures					Flow Inputs		AM LOS Performance Measures					PM LOS Performance Measures									
	Length (ft)	Number of Lanes (N)	Interchange Density (I/mi)	AM Peak	PM Peak	V _p	FFS	S	D	LOS	V _p	FFS	S	D	LOS	AM Peak	PM Peak	V _p	FFS	S	D	LOS	V _p	FFS	S	D	LOS					
				(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	(veh/h)	(veh/h)	(pc/h/ln)	(mi/h)	(mi/h)	(pc/mi/ln)	
Eastbound	West of El Dorado Hills Blvd	8180	4	0.67	3,683	6,274	1030.84	73.11	75	74.9895	13.746	B	1756.038	73.11	75	68.6725	25.571	C	3,713	6,366	1039.24	73.11	75	74.983	13.86	B	1781.788	73.11	75	68.2341	26.113	D
	Latrobe Rd NB Off Ramp to Latrobe Rd On Ramp	1490	4	0.67	1,615	3,820	452.024	73.11	75	71.6759	6.3065	A	1069.185	73.11	75	74.947	14.266	B	1,630	3,866	456.223	73.11	75	71.7267	6.3606	A	1082.06	73.11	75	74.9255	14.442	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy SB On Ramp	1130	4	0.50	1,965	4,300	549.986	73.6	75	72.7582	7.5591	A	1203.533	73.6	75	74.5414	16.146	B	1,965	4,300	549.986	73.6	75	72.7582	7.5591	A	1203.533	73.6	75	74.5414	16.146	B
	East of Silva Valley Pkwy	1980	4	0.50	3,348	5,891	937.076	73.6	75	74.9562	12.502	B	1648.84	73.6	75	70.3396	23.441	C	3,376	5,909	944.913	73.6	75	74.9664	12.604	B	1653.878	73.6	75	70.267	23.537	C
Westbound	East of Silva Valley Pkwy	2950	4	0.50	6,579	4,245	1841.4	73.6	75	67.1629	27.417	D	1188.139	73.6	75	74.6082	15.925	B	6,589	4,275	1844.2	73.6	75	67.1106	27.48	D	1196.535	73.6	75	74.5724	16.045	B
	Silva Valley Pkwy Off Ramp to Silva Valley Pkwy NB On Ramp	1640	3	0.50	5,325	2,805	1987.23	73.6	75	64.211	30.948	D	1046.793	73.6	75	74.9758	13.962	B	5,325	2,805	1987.23	73.6	75	64.211	30.948	D	1046.793	73.6	75	74.9758	13.962	B
	El Dorado Hills Blvd Off Ramp to El Dorado Hills Blvd On Ramp	3080	3	0.67	5,377	2,888	2006.63	73.11	75	63.7826	31.461	D	1077.768	73.11	75	74.933	14.383	B	5,419	2,915	2022.31	73.11	75	63.4306	31.882	D	1087.844	73.11	75	74.9146	14.521	B
Universal Inputs:																																
PHF				0.92																												
P ₁				6%																												
P _{1V}				0.970873786																												

Merge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

																	Cumulative			
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures													
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _A)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v _D	v _F	v _R	v _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/ln)						(pc/mi/ln)		
G	Silva Valley Pkwy SB On Ramp	4	1	200	2557	1965	592	2863	2200	663	63	0.1987	437.06	9600	881	328	880	0.2982	15.95	B
H	Silva Valley Pkwy NB On Ramp	4	1	1000	3348	2557	791	3748	2863	886	82	0.1071	306.61	9600	1278	230	1145	0.3904	14.637	B
Universal Inputs:																				
	Length	1500	(ft)																	
	S _{FF}	70	(mi/h)																	
	S _{FR}	35	(mi/h)																	
	PHF	0.92																		
	(P _r)	6%																		
	f _{HV}	0.970873786																		

Merge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

Segment Inputs				Conditions																
				PM Flow Inputs			PM LOS Performance Measures													
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _A)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v _D	v _F	v _R	v _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
																			(veh/h)	(veh/h)
	(N)		(ft)																	
G	Silva Valley Pkwy SB On Ramp	4	1	200	5148	4300	848	5764	4814	949	138	0.0991	477.21	9600	2168	358	1926	0.6004	26.21	C
H	Silva Valley Pkwy NB On Ramp	4	1	1000	5891	5148	743	6595	5764	832	165	0.1138	656.01	9600	2554	492	2305	0.687	23.293	C
Universal Inputs:																				
Length 1500 (ft)																				
S _{FF} 70 (mi/h)																				
S _{FR} 35 (mi/h)																				
PHF 0.92																				
(P _T) 6%																				
f _{HV} 0.970873786																				

Diverge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

													Cumulative C					
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures											
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume	Upstream Volume	Ramp Volume	v _D	v _F	v _R	P _{FD}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS	
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)					(pc/mi/ln)		
B	4	1	140	2077	3683	1606	2325.34	4123.4	1798	0.436	2811.9	9600	656	2109	2812	0.4295	27.174	C
C	4	1	120	1615	2077	462	1808.1	2325.3	517.24	0.436	1305.6	9600	510	979	1306	0.2422	14.4	B
Universal Inputs:																		
Leng 1500 (ft)																		
S _{FF} 70 (mi/h)																		
S _{FR} 35 (mi/h)																		
PHF 0.92																		
(P _T) 6%																		
f _{HV} 0.970873786																		

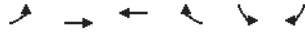
Diverge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

Segment Inputs		Conditions					PM LOS Performance Measures												
		Number of Lanes (N)	Number of Ramp Lanes	Length of Deceleration Lane (L _D) (ft)	Downstream Volume (D) (veh/h)	Upstream Volume (F) (veh/h)	Ramp Volume (R) (veh/h)	V _D (pc/h/ln)	V _F (pc/h/ln)	V _R (pc/h/ln)	P _{FD}	V ₁₂ (pc/h/ln)	Capacity	V ₃	V _{12a}	v/c	D (pc/mi/ln)	LOS	
B	Latrobe SB Off Ramp	4	1	140	4851	6274	1423	5431.01	7024.2	1593.1	0.436	3961.1	9600	1532	2971	3961	0.7317	37.057	E
C	Latrobe NB Off Ramp	4	1	120	3820	4851	1031	4276.74	5431	1154.3	0.436	3018.9	9600	1206	2264	3019	0.5657	29.135	D
Universal Inputs:																			
	Leng	1500		(ft)															
	S _{FF}	70		(mi/h)															
	S _{FR}	35		(mi/h)															
	PHF	0.92																	
	(P _T)	6%																	
	f _{HV}	0.970873786																	

Dixon Ranch
5: Green Valley Rd & Wilson Estates

75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Volume (veh/h)	10	357	758	5	12	27
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	11	388	824	5	13	29
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	829				1236	827
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	829				1236	827
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				93	92
cM capacity (veh/h)	802				192	372
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	399	829	42			
Volume Left	11	0	13			
Volume Right	0	5	29			
cSH	802	1700	288			
Volume to Capacity	0.01	0.49	0.15			
Queue Length 95th (ft)	1	0	13			
Control Delay (s)	0.4	0.0	19.6			
Lane LOS	A		C			
Approach Delay (s)	0.4	0.0	19.6			
Approach LOS			C			
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization		50.2%		ICU Level of Service	A	
Analysis Period (min)		15				

Dixon Ranch
6: Green Valley Rd & Malcom Dixon Rd

75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM



Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↔	↔		↔	
Volume (veh/h)	4	338	724	5	10	25
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	367	787	5	11	27
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	792				1166	790
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	792				1166	790
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	99				95	93
cM capacity (veh/h)	828				213	390
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	372	792	38			
Volume Left	4	0	11			
Volume Right	0	5	27			
cSH	828	1700	315			
Volume to Capacity	0.01	0.47	0.12			
Queue Length 95th (ft)	0	0	10			
Control Delay (s)	0.2	0.0	18.0			
Lane LOS	A		C			
Approach Delay (s)	0.2	0.0	18.0			
Approach LOS			C			
Intersection Summary						
Average Delay			0.6			
Intersection Capacity Utilization		48.4%		ICU Level of Service	A	
Analysis Period (min)		15				

Dixon Ranch
7: Deer Valley Rd. & Green Valley Rd.

75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔		↔	↔			↕		↔	↔	
Volume (veh/h)	5	314	1	2	522	6	6	0	10	42	0	11
Sign Control		Free			Free			Stop			Stop	
Grade		0%			0%			0%			0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	5	341	1	2	567	7	7	0	11	46	0	12
Pedestrians												
Lane Width (ft)												
Walking Speed (ft/s)												
Percent Blockage												
Right turn flare (veh)												
Median type	None					None						
Median storage (veh)												
Upstream signal (ft)												
pX, platoon unblocked												
vC, conflicting volume	574			342			936	931	342	938	928	571
vC1, stage 1 conf vol												
vC2, stage 2 conf vol												
vCu, unblocked vol	574			342			936	931	342	938	928	571
tC, single (s)	4.1			4.1			7.1	6.5	6.2	7.1	6.5	6.2
tC, 2 stage (s)												
tF (s)	2.2			2.2			3.5	4.0	3.3	3.5	4.0	3.3
p0 queue free %	99			100			97	100	98	81	100	98
cM capacity (veh/h)	999			1217			238	265	701	239	266	521
Direction, Lane #	EB 1	EB 2	WB 1	WB 2	NB 1	SB 1						
Volume Total	5	342	2	574	17	58						
Volume Left	5	0	2	0	7	46						
Volume Right	0	1	0	7	11	12						
cSH	999	1700	1217	1700	405	270						
Volume to Capacity	0.01	0.20	0.00	0.34	0.04	0.21						
Queue Length 95th (ft)	0	0	0	0	3	20						
Control Delay (s)	8.6	0.0	8.0	0.0	14.3	21.9						
Lane LOS	A		A		B	C						
Approach Delay (s)	0.1		0.0		14.3	21.9						
Approach LOS					B	C						
Intersection Summary												
Average Delay			1.6									
Intersection Capacity Utilization			40.3%	ICU Level of Service	A							
Analysis Period (min)			15									

Dixon Ranch
8: Silver Springs Pkwy & Green Valley Rd

75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↔		↔	↔	↔	↔
Volume (vph)	628	29	16	838	74	32
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0			4.0	4.0	
Lane Util. Factor	1.00			1.00	1.00	
Frt	0.99			1.00	0.96	
Flt Protected	1.00			1.00	0.97	
Satd. Flow (prot)	1851			1861	1726	
Flt Permitted	1.00			0.99	0.97	
Satd. Flow (perm)	1851			1837	1726	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Adj. Flow (vph)	683	32	17	911	80	35
RTOR Reduction (vph)	3	0	0	0	29	0
Lane Group Flow (vph)	712	0	0	928	86	0
Turn Type	NA		Perm	NA	NA	
Protected Phases	4			8	2	
Permitted Phases	8					
Actuated Green, G (s)	28.0			28.0	8.0	
Effective Green, g (s)	28.0			28.0	8.0	
Actuated g/C Ratio	0.64			0.64	0.18	
Clearance Time (s)	4.0			4.0	4.0	
Vehicle Extension (s)	3.0			3.0	3.0	
Lane Grp Cap (vph)	1177			1169	313	
v/s Ratio Prot	0.38				c0.05	
v/s Ratio Perm				c0.51		
w/c Ratio	0.61			0.79	0.28	
Uniform Delay, d1	4.7			5.9	15.5	
Progression Factor	1.00			1.00	1.00	
Incremental Delay, d2	0.9			3.8	0.5	
Delay (s)	5.6			9.7	16.0	
Level of Service	A			A	B	
Approach Delay (s)	5.6			9.7	16.0	
Approach LOS	A			A	B	
Intersection Summary						
HCM 2000 Control Delay			8.4	HCM 2000 Level of Service	A	
HCM 2000 Volume to Capacity ratio			0.68			
Actuated Cycle Length (s)			44.0	Sum of lost time (s)	8.0	
Intersection Capacity Utilization			69.6%	ICU Level of Service	C	
Analysis Period (min)			15			
c Critical Lane Group						

Dixon Ranch 75-25 Modified Model, Cumulative Plus Project
 11: Cameron Park Dr. & Green Valley Rd. Timing Plan: AM


Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Volume (vph)	27	144	313	190	213	12	300	39	137	15	115	24
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Frt	1.00	0.90	1.00	0.99	1.00		1.00	0.88		1.00	0.97	
Flt Protected	0.95	1.00	0.95	1.00	0.95		1.00	1.00		0.95	1.00	
Satd. Flow (prot)	1770	1672	1770	1848	1770		1645	1770		1815	1770	
Flt Permitted	0.95	1.00	0.95	1.00	0.95		1.00	1.00		0.95	1.00	
Satd. Flow (perm)	1770	1672	1770	1848	1770		1645	1770		1815	1770	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92		0.92	0.92		0.92	0.92	
Adj. Flow (vph)	29	157	340	207	232		13	326		42	149	
RTOR Reduction (vph)	0	83	0	0	2		0	93		0	9	
Lane Group Flow (vph)	29	414	0	207	243		0	326		98	0	
Turn Type	Prot	NA	Prot	NA	Prot		NA	Prot		NA	Prot	
Protected Phases	7	4	3	8	5		2	1		6		
Permitted Phases												
Actuated Green, G (s)	1.9	25.4	11.9	35.4	18.0		32.1	0.7		14.8		
Effective Green, g (s)	1.9	25.4	11.9	35.4	18.0		32.1	0.7		14.8		
Actuated g/C Ratio	0.02	0.30	0.14	0.41	0.21		0.37	0.01		0.17		
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0		4.0	4.0		4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0		3.0	3.0		3.0	3.0	
Lane Grp Cap (vph)	39	493	244	759	370		613	14		311		
v/s Ratio Prot	0.02	c0.25	c0.12	0.13	c0.18		0.06	0.01		c0.08		
v/s Ratio Perm												
v/c Ratio	0.74	0.84	0.85	0.32	0.88		0.16	1.14		0.46		
Uniform Delay, d1	41.9	28.4	36.2	17.2	33.0		18.0	42.7		32.0		
Progression Factor	1.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00	
Incremental Delay, d2	54.3	11.9	23.0	0.2	20.9		0.1	291.3		1.1		
Delay (s)	96.1	40.4	59.2	17.4	53.9		18.1	334.0		33.1		
Level of Service	F	D	E	B	D		B	F		C		
Approach Delay (s)	43.4		36.6		40.7			61.9				
Approach LOS	D		D		D			E				
Intersection Summary												
HCM 2000 Control Delay	42.6		HCM 2000 Level of Service				D					
HCM 2000 Volume to Capacity ratio	0.77											
Actuated Cycle Length (s)	86.1		Sum of lost time (s)				16.0					
Intersection Capacity Utilization	74.8%		ICU Level of Service				D					
Analysis Period (min)	15											
c Critical Lane Group												

Dixon Ranch 75-25 Modified Model, Cumulative Plus Project
 12: El Dorado Hills Blvd. & Francisco Dr. Timing Plan: AM

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔	↔
Sign Control	Stop			Stop			Stop			Stop		
Volume (vph)	4	41	501	47	56	55	436	238	43	142	575	11
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	4	45	545	51	61	60	474	259	47	154	625	12
Direction, Lane #												
Volume Total (vph)	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Left (vph)	4	0	51	474	0	154	0					
Volume Right (vph)	0	545	60	0	47	0	12					
Hadj (s)	0.05	-0.57	-0.12	0.53	-0.07	0.53	0.02					
Departure Headway (s)	7.7	3.2	7.0	6.9	6.3	7.1	6.5					
Degree Utilization, x	0.10	0.48	0.33	0.90	0.53	0.30	1.16					
Capacity (veh/h)	440	1116	499	511	568	497	554					
Control Delay (s)	11.6	9.2	13.4	44.3	14.9	11.9	111.7					
Approach Delay (s)	9.4	13.4		32.8		92.2						
Approach LOS	A	B		D		F						
Intersection Summary												
Delay	45.5											
Level of Service	E											
Intersection Capacity Utilization	80.9%		ICU Level of Service				D					
Analysis Period (min)	15											

Dixon Ranch
24: Silva Valley Pkwy. & Appian Way


75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	39	1	92	158	2	63	25	272	52	30	376	25
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	42	1	100	172	2	68	27	296	57	33	409	27
Direction, Lane #	EB 1	WB 1	NB 1	SB 1								
Volume Total (vph)	143	242	379	468								
Volume Left (vph)	42	172	27	33								
Volume Right (vph)	100	68	57	27								
Hadj (s)	-0.33	0.01	-0.04	0.01								
Departure Headway (s)	6.9	6.9	6.2	6.1								
Degree Utilization, x	0.27	0.46	0.65	0.79								
Capacity (veh/h)	439	470	548	567								
Control Delay (s)	12.5	15.7	20.0	28.1								
Approach Delay (s)	12.5	15.7	20.0	28.1								
Approach LOS	B	C	C	D								
Intersection Summary												
Delay	21.3											
Level of Service	C											
Intersection Capacity Utilization	55.7%		ICU Level of Service	B								
Analysis Period (min)	15											

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.

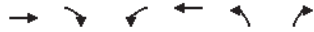
75-25 Modified Model, Cumulative Plus Project
Timing Plan: AM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↕			↕		↕
Volume (veh/h)	295	39	0	757	0	21
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	321	42	0	823	0	23
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)	796					
pX, platoon unblocked				0.72		
vC, conflicting volume				363	1165	342
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol				363	1037	342
tC, single (s)				4.1	6.4	6.2
tC, 2 stage (s)						
tF (s)				2.2	3.5	3.3
p0 queue free %				100	100	97
cM capacity (veh/h)				1196	185	701
Direction, Lane #	EB 1	WB 1	NB 1			
Volume Total	363	823	23			
Volume Left	0	0	0			
Volume Right	42	0	23			
cSH	1700	1700	701			
Volume to Capacity	0.21	0.48	0.03			
Queue Length 95th (ft)	0	0	3			
Control Delay (s)	0.0	0.0	10.3			
Lane LOS				B		
Approach Delay (s)	0.0	0.0	10.3			
Approach LOS				B		
Intersection Summary						
Average Delay				0.2		
Intersection Capacity Utilization	43.2%		ICU Level of Service	A		
Analysis Period (min)	15					

Dixon Ranch
4: Loch Way & Green Valley Rd

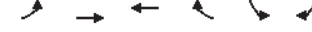
75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	↕	↕		↕	↕	↕
Volume (veh/h)	771	177	83	407	132	83
Sign Control	Free			Free	Stop	
Grade	0%			0%	0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	838	192	90	442	143	90
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type	None			None		
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume			1030		1557	934
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol			1030		1557	934
tC, single (s)			4.1		6.4	6.2
tC, 2 stage (s)						
tF (s)			2.2		3.5	3.3
p0 queue free %			87		0	72
cM capacity (veh/h)			674		107	322
Direction, Lane #	EB 1	WB 1	NB 1	NB 2		
Volume Total	1030	533	143	90		
Volume Left	0	90	143	0		
Volume Right	192	0	0	90		
cSH	1700	674	107	322		
Volume to Capacity	0.61	0.13	1.34	0.28		
Queue Length 95th (ft)	0	12	248	28		
Control Delay (s)	0.0	3.6	275.0	20.5		
Lane LOS		A	F	C		
Approach Delay (s)	0.0	3.6	176.7			
Approach LOS			F			
Intersection Summary						
Average Delay			24.0			
Intersection Capacity Utilization		94.7%		ICU Level of Service	F	
Analysis Period (min)		15				


Dixon Ranch
5: Green Valley Rd & Wilson Estates

75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM




Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations		↕	↕		↕	↕
Volume (veh/h)	27	855	482	16	8	19
Sign Control		Free	Free		Stop	
Grade		0%	0%		0%	
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	29	929	524	17	9	21
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type		None	None			
Median storage (veh)						
Upstream signal (ft)						
pX, platoon unblocked						
vC, conflicting volume	541				1521	533
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	541				1521	533
tC, single (s)	4.1				6.4	6.2
tC, 2 stage (s)						
tF (s)	2.2				3.5	3.3
p0 queue free %	97				93	96
cM capacity (veh/h)	1027				127	547
Direction, Lane #	EB 1	WB 1	SB 1			
Volume Total	959	541	29			
Volume Left	29	0	9			
Volume Right	0	17	21			
cSH	1027	1700	276			
Volume to Capacity	0.03	0.32	0.11			
Queue Length 95th (ft)	2	0	9			
Control Delay (s)	0.8	0.0	19.6			
Lane LOS	A		C			
Approach Delay (s)	0.8	0.0	19.6			
Approach LOS			C			
Intersection Summary						
Average Delay			0.9			
Intersection Capacity Utilization		76.8%		ICU Level of Service	D	
Analysis Period (min)		15				

Dixon Ranch
12: El Dorado Hills Blvd. & Francisco Dr. 75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕	↕		↕			↕		↕	↕	
Sign Control		Stop			Stop			Stop			Stop	
Volume (vph)	0	33	536	31	30	47	552	578	30	11	321	3
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly flow rate (vph)	0	36	583	34	33	51	600	628	33	12	349	3
Direction, Lane #	EB 1	EB 2	WB 1	NB 1	NB 2	SB 1	SB 2					
Volume Total (vph)	36	583	117	600	661	12	352					
Volume Left (vph)	0	0	34	600	0	12	0					
Volume Right (vph)	0	583	51	0	33	0	3					
Hadj (s)	0.03	-0.57	-0.17	0.53	0.00	0.53	0.03					
Departure Headway (s)	7.0	3.2	6.5	6.1	5.5	7.2	6.7					
Degree Utilization, x	0.07	0.52	0.21	1.01	1.02	0.02	0.65					
Capacity (veh/h)	479	1117	530	600	661	492	526					
Control Delay (s)	10.6	9.6	11.3	63.2	61.3	9.2	20.1					
Approach Delay (s)	9.6		11.3	62.2	19.8							
Approach LOS	A		B	F	C							
Intersection Summary												
Delay			39.4									
Level of Service			E									
Intersection Capacity Utilization			70.5%		ICU Level of Service		C					
Analysis Period (min)			15									


Dixon Ranch
13: El Dorado Hills Blvd. & Harvard Way 75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM



Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	↕	↕	↕	↕	↕	↕	
Volume (vph)	353	100	1100	461	123	702	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	
Total Lost time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Lane Util. Factor	1.00	1.00	0.95	1.00	0.97	0.95	
Frt	1.00	0.85	1.00	0.85	1.00	1.00	
Flt Protected	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (prot)	1770	1583	3539	1583	3433	3539	
Flt Permitted	0.95	1.00	1.00	1.00	0.95	1.00	
Satd. Flow (perm)	1770	1583	3539	1583	3433	3539	
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	384	109	1196	501	134	763	
RTOR Reduction (vph)	0	78	0	279	0	0	
Lane Group Flow (vph)	384	31	1196	222	134	763	
Turn Type	NA	Perm	NA	Perm	Prot	NA	
Protected Phases	8		2		1	6	
Permitted Phases		8		2			
Actuated Green, G (s)	15.7	15.7	24.3	24.3	2.9	31.2	
Effective Green, g (s)	15.7	15.7	24.3	24.3	2.9	31.2	
Actuated g/C Ratio	0.29	0.29	0.44	0.44	0.05	0.57	
Clearance Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Vehicle Extension (s)	3.0	3.0	3.0	3.0	3.0	3.0	
Lane Grp Cap (vph)	506	452	1566	700	181	2011	
v/s Ratio Prot	c0.22		c0.34		c0.04	0.22	
v/s Ratio Perm		0.02		0.14			
v/c Ratio	0.76	0.07	0.76	0.32	0.74	0.38	
Uniform Delay, d1	17.9	14.3	12.9	9.9	25.6	6.5	
Progression Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Incremental Delay, d2	6.4	0.1	2.3	0.3	15.0	0.1	
Delay (s)	24.3	14.3	15.2	10.2	40.6	6.6	
Level of Service	C	B	B	B	D	A	
Approach Delay (s)	22.1		13.7		11.7		
Approach LOS	C		B		B		
Intersection Summary							
HCM 2000 Control Delay			14.5		HCM 2000 Level of Service		B
HCM 2000 Volume to Capacity ratio			0.76				
Actuated Cycle Length (s)			54.9		Sum of lost time (s)		12.0
Intersection Capacity Utilization			63.5%		ICU Level of Service		B
Analysis Period (min)			15				
c Critical Lane Group							

Dixon Ranch
25: Site Dwy RIRO & Green Valley Rd.


75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM



Movement	EBT	EBR	WBL	WBT	NBL	NBR	
Lane Configurations	↕			↕		↕	
Volume (veh/h)	729	117	0	506	0	14	
Sign Control	Free			Free	Stop		
Grade	0%			0%	0%		
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	
Hourly flow rate (vph)	792	127	0	550	0	15	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type	None		None				
Median storage (veh)							
Upstream signal (ft)	836						
pX, platoon unblocked				0.92			
vC1, conflicting volume			920	1406	856		
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol			920	1397	856		
tC, single (s)			4.1	6.4	6.2		
tC, 2 stage (s)							
tF (s)			2.2	3.5	3.3		
p0 queue free %			100	100	96		
cM capacity (veh/h)			742	142	357		
Direction, Lane #	EB 1	WB 1	NB 1				
Volume Total	920	550	15				
Volume Left	0	0	0				
Volume Right	127	0	15				
cSH	1700	1700	357				
Volume to Capacity	0.54	0.32	0.04				
Queue Length 95th (ft)	0	0	3				
Control Delay (s)	0.0	0.0	15.5				
Lane LOS			C				
Approach Delay (s)	0.0	0.0	15.5				
Approach LOS			C				
Intersection Summary							
Average Delay			0.2				
Intersection Capacity Utilization			55.5%	ICU Level of Service		B	
Analysis Period (min)	15						

Dixon Ranch
26: Site Dwy. Full/Site Dwy. & Green Valley Rd.

75-25 Modified Model, Cumulative Plus Project
Timing Plan: PM



Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations	↕	↕		↕	↕			↕	↕		↕	↕	
Volume (vph)	0	626	117	70	367	0	139	0	28	0	0	0	
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Total Lost time (s)				4.0	4.0				4.0	4.0			
Lane Util. Factor				1.00	1.00				1.00	1.00			
Frt				0.98	1.00				1.00	0.85			
Flt Protected				1.00	0.95				0.95	1.00			
Satd. Flow (prot)				1819	1770				1770	1583			
Flt Permitted				1.00	0.19				0.76	1.00			
Satd. Flow (perm)				1819	358				1410	1583			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
Adj. Flow (vph)	0	680	127	76	399	0	151	0	30	0	0	0	
RTOR Reduction (vph)	0	12	0	0	0	0	0	0	23	0	0	0	
Lane Group Flow (vph)	0	795	0	76	399	0	0	151	7	0	0	0	
Turn Type	Perm	NA		Perm	NA		Perm	NA	Perm	Perm			
Protected Phases			6			2			4				
Permitted Phases	6			2			4			8			
Actuated Green, G (s)	23.4			23.4			23.4			10.4			
Effective Green, g (s)	23.4			23.4			23.4			10.4			
Actuated g/C Ratio	0.56			0.56			0.56			0.25			
Clearance Time (s)	4.0			4.0			4.0			4.0			
Vehicle Extension (s)	3.0			3.0			3.0			3.0			
Lane Grp Cap (vph)	1018			200			1042			350			
v/s Ratio Prot	c0.44						0.21						
v/s Ratio Perm				0.21						c0.11			
v/c Ratio	0.78			0.38			0.38			0.43			
Uniform Delay, d1	7.2			5.1			5.2			13.2			
Progression Factor	1.00			1.00			1.00			1.00			
Incremental Delay, d2	3.9			1.2			0.2			0.9			
Delay (s)	11.1			6.4			5.4			14.1			
Level of Service	B			A			A			B			
Approach Delay (s)	11.1			5.5			13.7			0.0			
Approach LOS	B			A			B			A			
Intersection Summary													
HCM 2000 Control Delay				9.6			HCM 2000 Level of Service			A			
HCM 2000 Volume to Capacity ratio				0.67									
Actuated Cycle Length (s)				41.8			Sum of lost time (s)			8.0			
Intersection Capacity Utilization				61.6%			ICU Level of Service			B			
Analysis Period (min)	15												
c Critical Lane Group													

Merge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

																	Cumulative plus Propo			
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures													
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _A)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v _D	v _F	v _R	v _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/h)	(pc/h)			(pc/h/ln)					(pc/mi/ln)			
G H	Silva Valley Pkwy SB On Ramp	4	1	200	2585	1965	620	2894	2200	694	63	0.1947	428.43	9600	886	321	880	0.3015	16.18	B
	Silva Valley Pkwy NB On Ramp	4	1	1000	3376	2585	791	3780	2894	886	83	0.1071	309.96	9600	1292	232	1158	0.3937	14.735	B
Universal Inputs:																				
Length 1500 (ft)																				
S _{FF} 70 (mi/h)																				
S _{FR} 35 (mi/h)																				
PHF 0.92																				
(P _r) 6%																				
f _{HV} 0.970873786																				

Merge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

Segment Inputs		sed Project Conditions																		
		PM Flow Inputs			PM LOS Performance Measures															
	Number of Lanes	Number of Ramp Lanes	Length of Acceleration Lane (L _A)	Downstream Volume (D)	Upstream Volume (F)	Ramp Volume (R)	v _D	v _F	v _R	v _F /S _{FR}	P _{FM}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS		
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h)	(pc/hr)	(pc/h)			(pc/h/ln)					(pc/mi/ln)			
G	Silva Valley Pkwy SB On Ramp	4	1	200	5166	4300	866	5784	4814	970	138	0.0966	465.08	9600	2175	349	1926	0.6025	26.358	C
H	Silva Valley Pkwy NB On Ramp	4	1	1000	5909	5166	743	6616	5784	832	165	0.1138	658.3	9600	2563	494	2313	0.6891	23.356	C
Universal Inputs:																				
Length		1500	(ft)																	
S _{FF}		70	(mi/h)																	
S _{FR}		35	(mi/h)																	
PHF		0.92																		
(P _T)		6%																		
f _{HV}		0.970873786																		

Diverge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

														Cumulative plus Propose				
Segment Inputs				AM Flow Inputs			AM LOS Performance Measures											
	Number of Lanes	Number of Ramp Lanes	Length of Deceleration Lane (L _D)	Downstream Volume	Upstream Volume	Ramp Volume	v _D	v _F	v _R	P _{FD}	v ₁₂	Capacity	v ₃	v _{12a}	v/c	D	LOS	
	(N)		(ft)	(veh/h)	(veh/h)	(veh/h)	(pc/h/ln)	(pc/h/ln)	(pc/h/ln)		(pc/h/ln)					(pc/mi/ln)		
B	4	1	140	2107	3713	1606	2358.92	4156.9	1798	0.436	2826.5	9600	665	2120	2827	0.433	27.3	C
C	4	1	120	1630	2107	477	1824.89	2358.9	534.03	0.436	1329.7	9600	515	997	1330	0.2457	14.607	B
Universal Inputs:																		
Leng 1500 (ft)																		
S _{FF} 70 (mi/h)																		
S _{FR} 35 (mi/h)																		
PHF 0.92																		
(P _T) 6%																		
f _{HV} 0.970873786																		

Diverge

75-25 Mod-Cumulative Basic Freeway Segments (State Route 50)

Segment Inputs		d Project Conditions																	
		PM Flow Inputs			PM LOS Performance Measures														
		Number of Lanes (N)	Number of Ramp Lanes	Length of Deceleration Lane (L _D) (ft)	Downstream Volume (D) (veh/h)	Upstream Volume (F) (veh/h)	Ramp Volume (R) (veh/h)	V _D (pc/h/ln)	V _F (pc/h/ln)	V _R (pc/h/ln)	P _{FD}	V ₁₂ (pc/h/ln)	Capacity	V ₃	V _{12a}	v/c	D	LOS	
B	Latrobe SB Off Ramp	4	1	140	4943	6366	1423	5534.01	7127.2	1593.1	0.436	4006	9600	1561	3004	4006	0.7424	37.443	E
C	Latrobe NB Off Ramp	4	1	120	3866	4943	1077	4328.24	5534	1205.8	0.436	3092.9	9600	1221	2320	3093	0.5765	29.771	D
Universal Inputs:																			
Leng 1500		(ft)																	
S _{FF} 70		(mi/h)																	
S _{FR} 35		(mi/h)																	
PHF 0.92																			
(P _T) 6%																			
f _{HV} 0.970873786																			